

# NCEL

## Technical Note

November 1988

By M. Jacoby

Sponsored by Marine Corps Research,  
Development and Acquisition Command

# PRESSURE COEFFICIENTS FOR BASIC TENSIONED-MEMBRANE STRUCTURE FORMS

**ABSTRACT** This report details FY85 wind tunnel tests on basic tensioned-membrane structure forms, data reduction and analysis, and results. Models of parallel and diagonally-arched structures were tested. Testing was performed at the James Forestal Laboratory wind tunnel, Princeton University, and the environment wind tunnel located at the Naval Civil Engineering Laboratory (NCEL). All data and results have been converted to pressure coefficient form to facilitate their use in wind load calculations. Results are presented in Appendixes A, B, and C. Appendixes A and B give average and peak section pressure coefficients arranged by model for different wind incident angles, respectively. Appendix C shows pressure coefficient contour plots arranged by model for different wind incident angles. The effects of varying length, height, wind incident angle, and cross section on parallel-arched structures were measured. Finally, an example wind load calculation using the results is contained in Appendix D.

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NAVAL CIVIL ENGINEERING LABORATORY PORT HUENEME CALIFORNIA 93043

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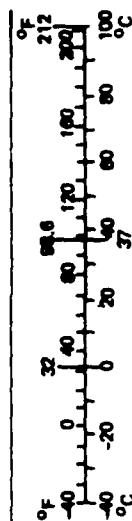
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# METRIC CONVERSION FACTORS

Approximate Conversions to Metric Measures			
Symbol	When You Know	Multiply by	To Find
in ft yd mi	inches	2.54	centimeters
	feet	30	centimeters
	yards	0.9	meters
	miles	1.6	kilometers
in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup> mi <sup>2</sup>	square inches	6.5	square centimeters
	square feet	0.09	square meters
	square yards	0.8	square meters
	square miles	2.6	square kilometers
oz lb	ounces	28	grams
	pounds	0.45	kilograms
	short tons	0.9	tonnes
	(2,000 lb)		
tsp Tbsp fl oz c pt qt gal ft <sup>3</sup> yd <sup>3</sup>	teaspoons	5	milliliters
	tablespoons	15	milliliters
	fluid ounces	30	milliliters
	cups	0.24	liters
	pints	0.47	liters
	quarts	0.95	liters
	gallons	3.8	liters
	cubic feet	0.03	cubic meters
	cubic yards	0.76	cubic meters
TEMPERATURE (exact)			
°F	Fahrenheit temperature	5/9 (after subtracting 32)	Celsius temperature
°C	Celsius temperature	9/5 (then add 32)	Fahrenheit temperature

\*1 in. = 2.54 (exactly). For other exact conversions and more detailed tables, see NBS Misc. Publ. 286, Units of Weights and Measures, Price \$2.25, SD Catalog No. C13.10.286.

Approximate Conversions from Metric Measures			
Symbol	When You Know	Multiply by	To Find
mm cm m km	millimeters	0.04	inches
	centimeters	0.4	inches
	meters	3.3	feet
	kilometers	1.1	yards
cm <sup>2</sup> m <sup>2</sup> km <sup>2</sup> ha	square centimeters	0.16	square inches
	square meters	1.2	square yards
	square kilometers	0.4	square miles
	hectares (10,000 m <sup>2</sup> )	2.5	acres
MASS (weight)			
g	grams	0.035	ounces
kg	kilograms	2.2	pounds
t	tonnes (1,000 kg)	1.1	short tons
VOLUME			
ml	milliliters	0.03	fluid ounces
l	liters	2.1	pints
l	liters	1.06	quarts
l	liters	0.26	gallons
m <sup>3</sup>	cubic meters	36	cubic feet
m <sup>3</sup>	cubic meters	1.3	cubic yards
TEMPERATURE (exact)			
°C	Celsius temperature	9/5 (then add 32)	Fahrenheit temperature



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PRESSURE COEFFICIENTS FOR BASIC TENSIONED-MEMBRANE  
STRUCTURE FORMS (Not Final) by M. Jacoby  
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1. Wind tunnel 2. Tensioned-Membrane I. C0078E-4-101

This report details FY85 wind tunnel tests on basic tensioned-membrane structure forms, data reduction and analysis, and results. Models of parallel and diagonally-arched structures were tested. Testing was performed at the James A. Forestal Laboratory wind tunnel, Princeton University, and the environment wind tunnel located at the Naval Civil Engineering Laboratory (NCEL). All data and results have been converted to pressure coefficient form to facilitate their use in wind load calculations. Results are presented in Appendixes A, B, and C. Appendixes A and B give average and peak section pressure coefficients arranged by model for different wind incident angles, respectively. Appendix C shows pressure coefficient contour plots arranged by model for different wind incident angles. The effects of varying length, height, wind incident angle, and cross section on parallel-arched structures were measured. Finally, an example wind load calculation using the results is contained in Appendix D.

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## INTRODUCTION

The Naval Civil Engineering Laboratory (NCEL) has been tasked by the Marine Corps to develop an expeditionary shelter system. This system will provide environmental protection for command and control, equipment maintenance and storage, and other combat support functions. The system being developed utilizes tensioned-membrane technology. Examples of tensioned-membrane structures are shown in Figures 1 and 2. This shelter system must withstand wind gusts up to 120 mph. For conventional buildings, standard guidelines such as NAVFAC DM-2.2 or ANSI A58.1-1982 (Ref 1 and 2) can be used to estimate wind loads. However, these guidelines cannot be used on tensioned-membrane structures due to their unusual construction. Section 6.4.3 of Reference 2 states that wind tunnel testing is "recommended for those buildings or structures having unusual geometric shapes, response characteristics, or site locations for which channeling effects or buffeting in the wake of upwind obstructions warrant special consideration, and for which no reliable documentation pertaining to wind effects is available in the literature."

FY84 development efforts included wind tunnel testing and compilation of data for various geometries and sizes of basic tensioned-membrane structure forms under consideration. The wind tunnel tests were performed by Ocean Structures, Inc., under contract to NCEL. Unreduced data from these tests were turned over to NCEL for reduction and analysis. This report documents these tests, FY85 verification wind tunnel testing performed by NCEL, analysis results, and conclusions.

## BACKGROUND THEORY

> This section details the basic equations and principles used for building aerodynamics and wind tunnel testing.

The pressure distribution around a body immersed in a moving fluid is primarily function of the local variation in fluid velocity produced by the body. ~~From Bernoulli's equation~~, with the subscript o referring to freestream conditions,

$$\frac{p_o}{\rho_o} + g z_o + \frac{1}{2} v_o^2 = \frac{p}{\rho} + g z + \frac{1}{2} v^2 \quad (1)$$

where  $p$  = static fluid pressure  
 $\rho$  = fluid mass density  
 $g$  = acceleration of gravity  
 $z$  = elevation  
 $v$  = fluid velocity

For the low velocities encountered in building aerodynamics, compressibility effects are negligible. Assuming constant elevation, Equation 1 reduces to,

$$p - p_o = \frac{1}{2} \rho_o (v_o^2 - v^2) \quad (2)$$

the maximum pressure difference from this equation is,

$$(p - p_o)_{\max} = \frac{1}{2} \rho_o v_o^2 \quad (3)$$

at the stagnation point where the flow velocity is zero. Dividing Equation 2 by this reference value,  $(1/2) \rho_o v_o^2$ , yields the following dimensionless form of Equation 2,

$$C_p = \frac{p - p_o}{\frac{1}{2} \rho_o v_o^2} = 1 - \left( \frac{v}{v_o} \right)^2 \quad (4)$$

where  $C_p$  is defined as the pressure coefficient. All data and results detailed here are in this form.

For wind tunnel testing of models, dynamic similitude conditions must be met. Dynamic similitude requires the Reynold's number for both the model and full size structure be the same, i.e.,

$$Re_{\text{model}} = Re_{\text{prototype}} \quad (5)$$

Strict adherence to Equation 5 is difficult when testing small-scale models. Generally, building forms are so angular that viscous effects are secondary. Reference 3 suggests that for wind tunnel testing of building forms, dynamic similitude will be met for model Reynold's numbers in excess of 11,000. As discussed later in this report, all model Reynold's numbers in this effort were between 170,000 and 950,000.

Finally, consideration must be given to accurately simulate full-scale boundary layer conditions. The distribution of mean windspeed with height is described by the power law relation,

$$\frac{u}{u} = \left( \frac{z}{\delta} \right)^a$$

where  $\mu_z$  = wind speed at height  $z$   
 $\mu$  = free stream wind speed  
 $z$  = height  
 $\delta$  = boundary layer thickness  
 $a$  = exponent, dependent on boundary layer type

Table 1 lists 1 different boundary layers and exponents.

Table 1. Boundary Layer Profiles and Power Law Exponents (Ref 4)

Boundary Layer Type	Power Law Exponent
City	0.34
Urban	0.18
Open terrain	0.17

Figure 3 shows these boundary layer profiles referenced to the NCEL wind tunnel. These boundary layers can be simulated in the wind tunnel by proper placement of flow impediments or screens upwind of the wind tunnel test section.

## TEST DESCRIPTIONS

Two separate series of wind tunnel tests are described in this section. The first series was performed by Ocean Structures, Inc., under contract to NCEL. After receipt of this data, and compilation of data on similar structures from other sources, verification testing was performed at NCEL.

Wind tunnel testing performed under contract to NCEL was performed at the low turbulence wind tunnel located at the James Forestal Laboratory, Princeton University. Table 2 lists the wind tunnel characteristics.

Table 2. Princeton Wind Tunnel Characteristics

Characteristic	Specification
Working cross section	3 ft by 5 ft
Maximum speed	120 mph
Maximum blockage	5%

Blockage refers to the ratio of maximum cross-sectional area to test section cross-sectional area. Pressure measurements were made with pressure taps connected to a 90-tube manometer board using dyed alcohol with a specific gravity of 0.793. All pressure measurements were



referenced to test section static pressure. Velocity measurements were made with a pitot-static tube connected to the manometer board. Figure 4 shows tube assignments.

Worst case environmental wind conditions are flow across open, flat terrain with few obstructions. Consequently, models were mounted away from test section walls to minimize boundary layer effects. Model mounting details are shown in Figure 5. The Reynold's number for all models varied between 570,000 and 950,000. Pressure taps were placed over roughly half of each model's surface. Each model was tested over wind incident angles from 0 to 180 degrees in 30-degree increments. Data were recorded by photographing the manometer board after the reading stabilized. Figure 6 shows a sample data photograph.

Dimensions of models tested at Princeton are shown in Figure 7. Models of both parallel- and diagonally-arched tensioned-membrane structures were tested. Model groups A, B, and C represent parallel-arched Clamshell Buildings, Inc., series 50 structures. Models in group A are of constant cross section and varying length. Models in groups B and C have constant lengths, but different cross sections (Figure 8). The diagonally-arched model tested was a 1/100-scale model of the Spandome, Inc., structure. All models were made of wood.

Another parallel-arched tensioned-membrane structure under consideration by NCEL was the Sprung Instant Structure, manufactured by Sprung Instant Structures, Inc. This structure is geometrically similar to the Clamshell structures mentioned above. Wind tunnel data on the Sprung was provided to NCEL by the manufacturer in FY84 (Ref 5). Preliminary analysis of wind tunnel data on both parallel-arched structures was conducted. Results showed much smaller negative pressures, or suctions, over the Sprung structure. Due to similarities between both parallel-arched structures, verification testing was performed at NCEL to rectify the differences. Characteristics of the NCEL wind tunnel are detailed in Table 3.

Table 3. NCEL Wind Tunnel  
Characteristics

Characteristic	Specification
Working cross section	3 ft by 5 ft
Maximum speed	45 mph
Maximum blockage	5 to 10%

Velocity measurements were made with a Kurtz hot-wire anemometer. Pressure measurements were made with pressure taps connected to a 47 channel Scanivalve Corp. valve tree and a Serta Systems 0 to 0.1 psid strain-gauge-type differential pressure transducer. All pressure measurements were referenced to tunnel test section static pressure, measured at the model roof height. All instrumentation was controlled by a Macsym 2 microcomputer. Real-time conversion of pressure measurements to pressure coefficient form was also performed with this system.

Figure 9 shows dimensions of two of the three models tested. The parallel-arched models are 1/60 scale. The Sprung Instant Structures model shown in Figure 9 will hereafter be referred to as model D. The diagonally-arched 1/100-scale Spandome model used earlier at Princeton was also tested (Figure 7).

The atmospheric boundary layer for open, flat terrain with scattered windbreaks was simulated. Figure 3 shows the actual boundary layer profile during testing. Flow velocities during testing were approximately 8.2 meters per second, resulting in test Reynold's numbers of 170,000. For the parallel-arched models, one quarter section was tapped. Measurements were taken for azimuth angles varying over 180 degrees, in 30-degree increments. During each test run, corresponding to each azimuth angle, every tap was sampled 6,000 times. Data output consisted of average, maximum, and minimum pressure coefficients for each tap.

## ANALYSIS

Comparisons of Princeton and NCEI wind tunnel data on the 1/100-scale diagonally-arched model were made. Results showed good correlation, with an average 3.7 percent difference between tap readings and a 26 percent standard deviation. Princeton test data were used for subsequent data analysis and reduction. Figure 10 shows comparisons of results on model D for NCEI, Reference 5 tests, and potential flow predictions. From the graphs, Reference 5 test data underestimated negative pressures, or suction. As a result, NCEI data on the Sprung Instant Structure model were used for analysis and reduction.

Figure 11 shows the analysis and reduction flowchart. Each major step is discussed below. The first step in the data reduction process consisted of conversion of Princeton data to pressure coefficient form. Data from the Princeton tests were recorded by photographing the manometer board after the readings had stabilized (see Figure 6 for example). Columns 9 and 10 as identified in Figure 4, recorded the total tunnel and test section static pressures referenced to atmospheric, respectively. The total tunnel pressure is the sum of the dynamic pressure, and the static pressure,

$$p_H = p_S + \frac{1}{2} \rho v^2 \quad (7)$$

where  $p_H$  = total tunnel pressure  
 $p_S$  = static pressure

The dynamic pressure is determined by subtracting the test section static pressure from the total tunnel pressure:

$$\frac{1}{2} \rho v^2 = p_H - p_S \quad (8)$$

This pressure difference can be calculated for the manometer board using,

$$p_H - p_s = \text{S.G. } \rho_{H_2O} (h_9 - h_{10}) \quad (9)$$

where S.G. = specific gravity of manometer fluid (0.793)

$\rho_{H_2O}$  = density of water

$h_9$  = manometer fluid height in column 9

$h_{10}$  = manometer fluid height in column 10

The pressure difference between the static pressure at tap x and test section static pressure is,

$$p_x - p_s = \text{S.G. } \rho_{H_2O} (h_x - h_{10}) \quad (10)$$

where  $h_x$  = manometer fluid height in column x.

At tap x, the pressure coefficient is calculated from,

$$C_p = \frac{h_x - h_{10}}{h_9 - h_{10}} \quad (11)$$

Using Equation 11, the pressure coefficients were calculated from the data photographs.

After data conversion to pressure coefficient form, pressure coefficient contour plots were generated. This was accomplished with the DISSPLA graphics software package from Issco, Inc., on the PRIME 750 minicomputer at NCEL. A geometric simulation of the wind tunnel models was constructed. Inputs were pressure tap coordinates and the correspondence pressure coefficients. Using a least squares weighting technique, contour plots were calculated in user defined intervals and superimposed on the model's surface. Appendix A contains top views of the computed contours, arranged by model and azimuth angle. These contours are approximations by nature of the weighting technique. An accuracy check revealed that the algorithm worked well in the interior of a surface, but had trouble accurately resolving contours along boundaries. This effect was compensated for by manually estimating pressure coefficient contours along model boundaries.

After contour plot generation, each model was sectioned, as shown in Figure 12. The parallel-arched models were divided up into 12 sections, symmetric about the model centerlines. The diagonally-arched model was divided into 16 sections. Section average pressure coefficients were calculated using,

$$\bar{C}_p = \frac{\sum_i C_{p_i} A_i}{\sum_i A_i} \quad (12)$$

where  $\bar{C}_p$  = section average pressure coefficient

$A_i$  =  $i^{th}$  area between contours

$C_{p_i}$  =  $i^{th}$  pressure coefficient, equal to the average to the coefficients defining  $A_i$

Areas were measured using a Salmoiraghi optical planimeter. Average section pressure coefficient results are found in Appendix B. Appendix C gives the largest negative or positive pressure coefficients for each section, arranged by model.

## CONCLUSION

Results detailed in this report are based on rigid models. With tensioned-membrane structures, structural shape is fabric dependent. The dynamic behavior and total deformation of these structures in heavy winds is unknown. Should these deformations be excessive, the resultant flow about the structures will be altered, and the results presented may not be accurate. For diagonally-arched structures, fabric flutter may be a problem due to large expanses of unconstrained fabric. This represents another dynamic phenomenon not accounted for in the present work. Finally, application of results presented in this report is demonstrated in Appendix D.

## ACKNOWLEDGMENT

The author would like to acknowledge Sophia K. Ashley of the Naval Civil Engineering Laboratory, for her experience, assistance, and support of the work described in this report.

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2. American National Standards Institute. ANSI A58.1-1982: American National Standard minimum design loads for buildings and other structures. New York, NY, Mar 1982.
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4. Naval Civil Engineering Laboratory. Technical Report R-912: Field and wind tunnel testing on natural ventilation cooling effects on three Navy buildings, by Sophia K. Ashley. Port Hueneme, CA, Dec 1984.

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Figure 1. Example of parallel-arched tensioned-membrane structure.



Figure 2. Example of diagonally-arched tensioned-membrane structure.

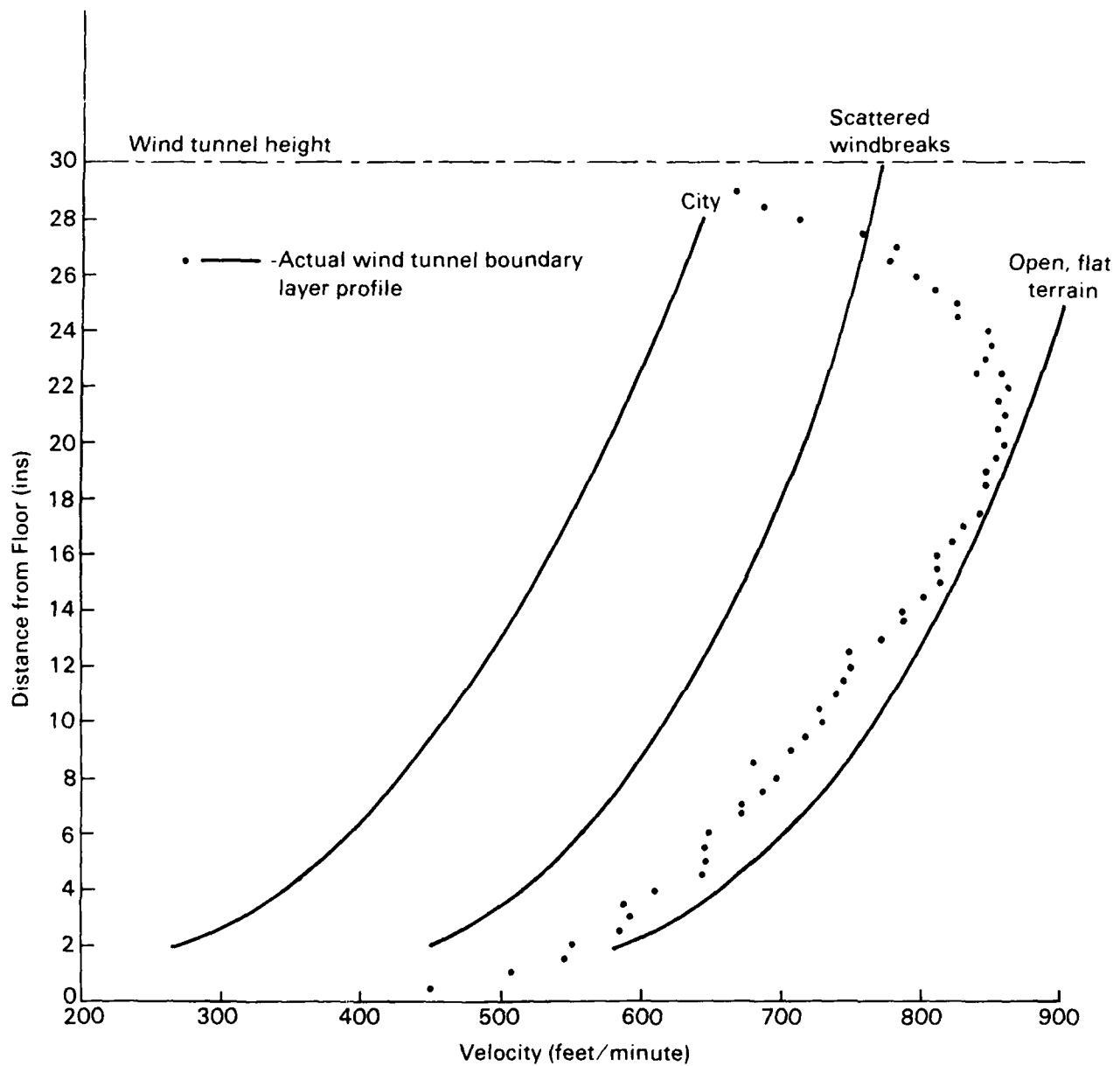


Figure 3. Atmospheric boundary layer profiles referenced to the NCEL wind tunnel.



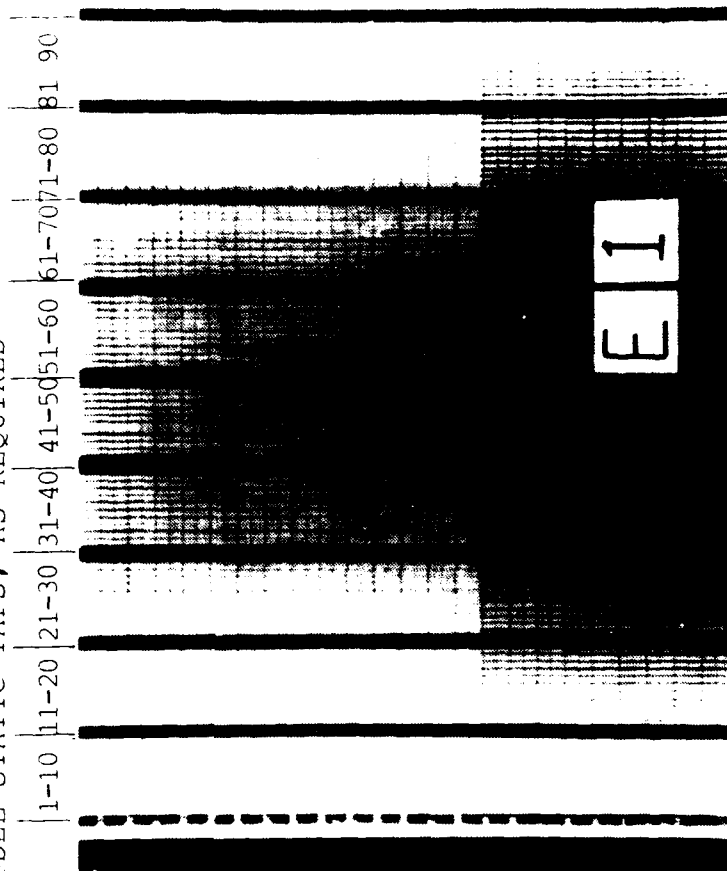
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Figure 4. Manometer board tube assignments for Princeton wind tunnel testing.

Details of Princeton  $3\frac{1}{2}' \times 5'$  wind tunnel test section.

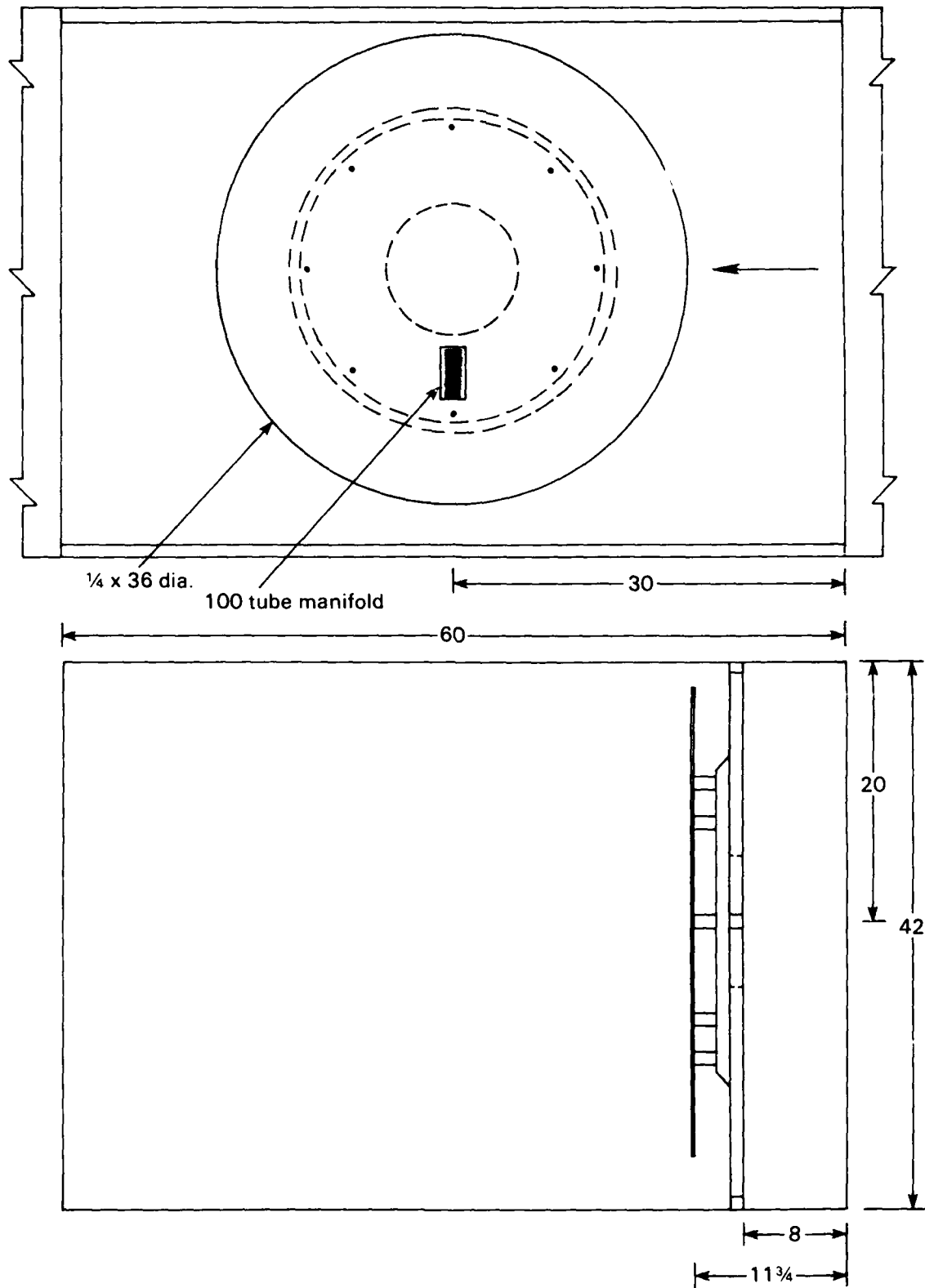


Figure 5. Princeton wind tunnel test section.

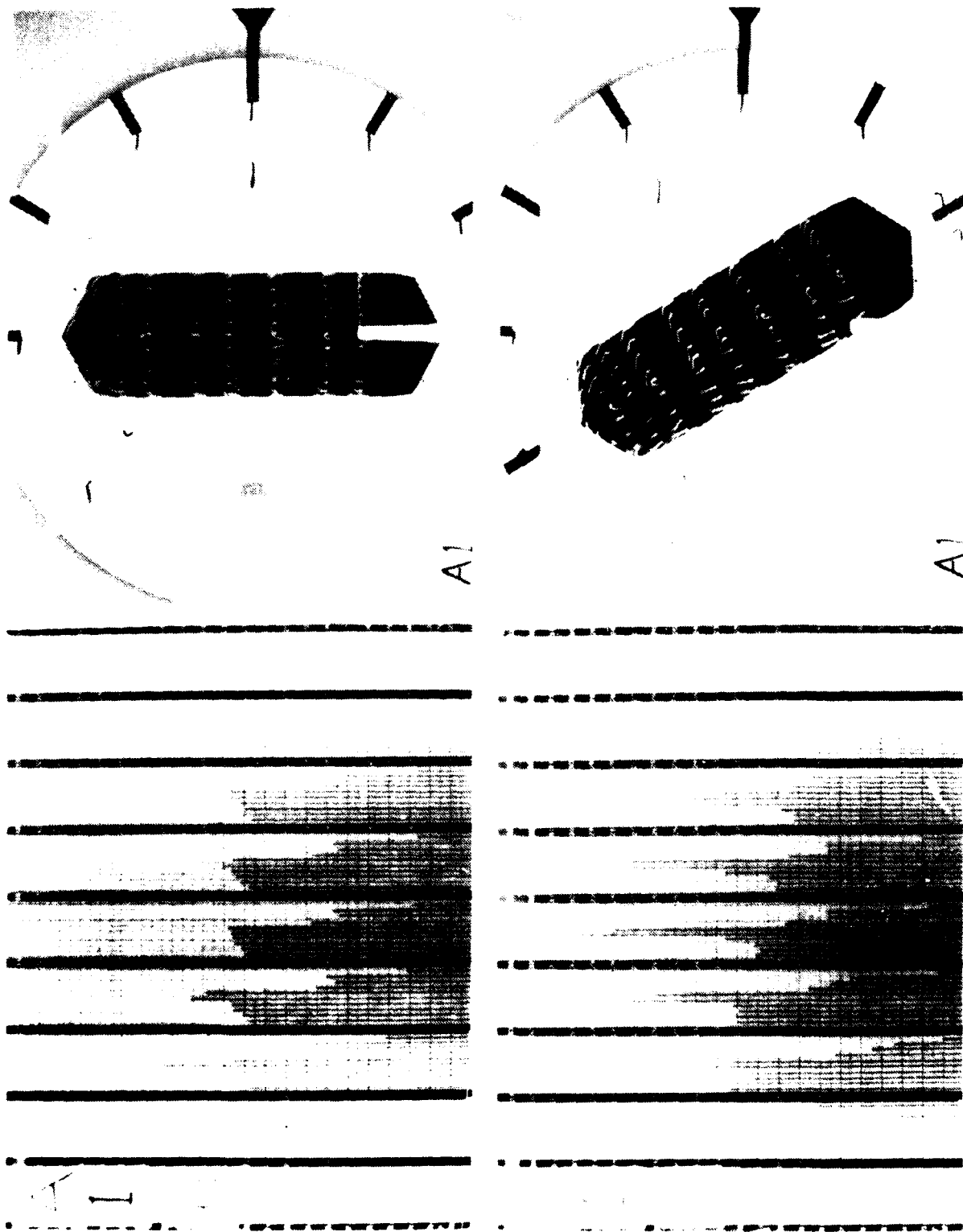


Figure 6. Sample of Princeton wind tunnel the data.

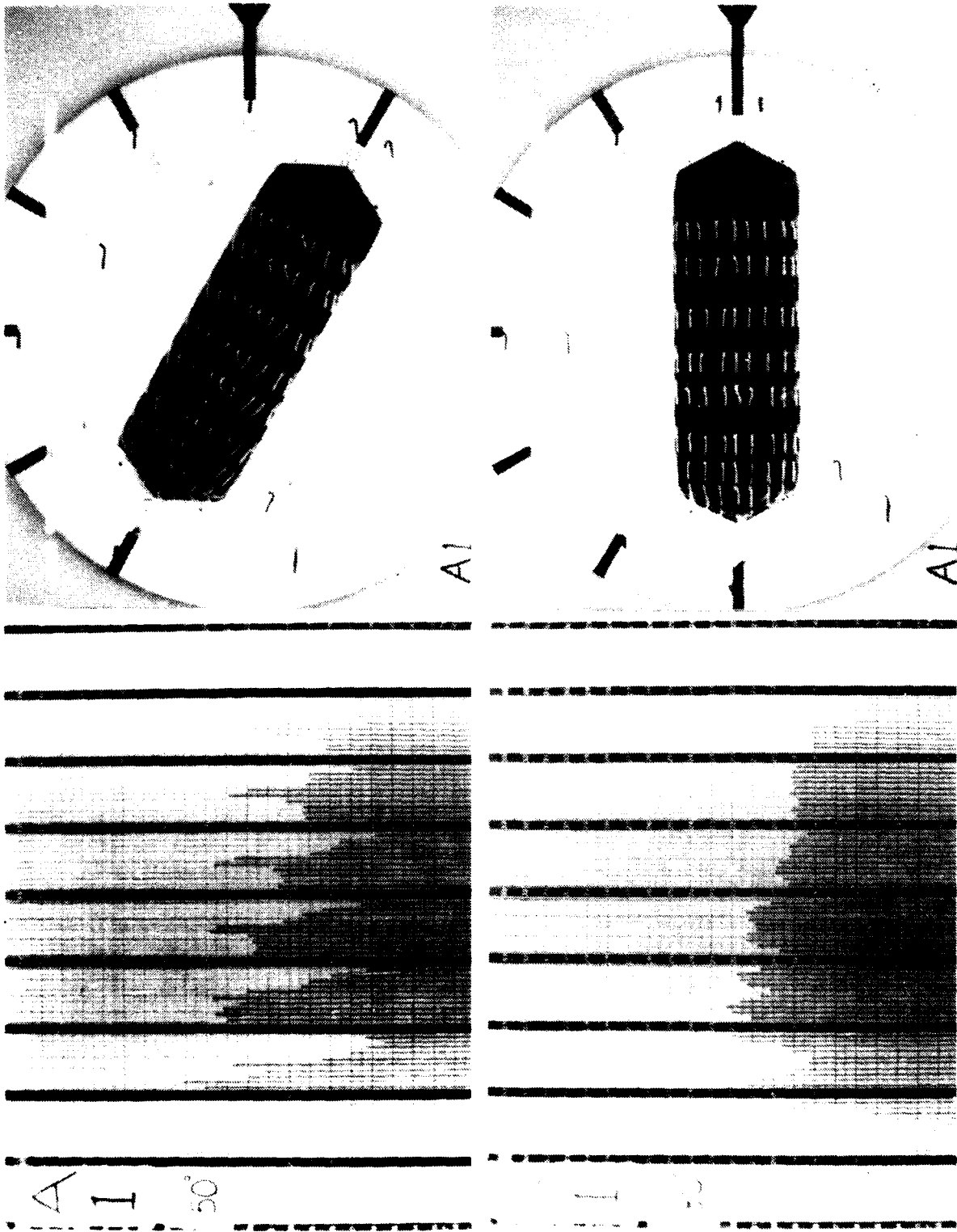
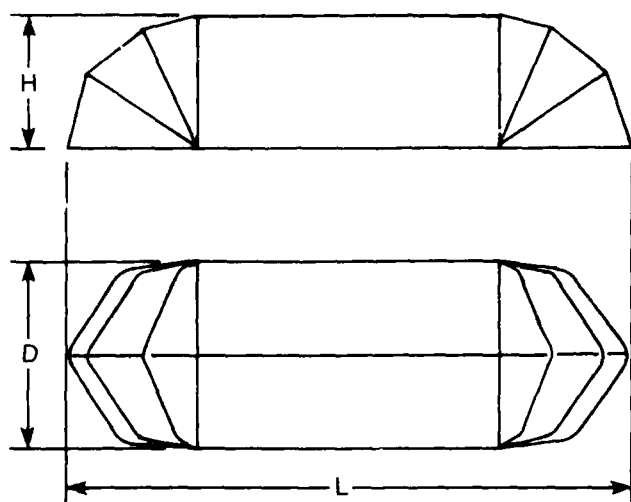


Figure 6. Continued.



Model	L *	D *	H *
A1	24.0	7.98	3.07
A3	18.0	7.98	3.07
A5	12.0	7.98	3.07
A6	9.0	7.98	3.07
B6	11.72	8.53	4.36
C6	14.30	9.08	5.65

\* All dimensions in inches

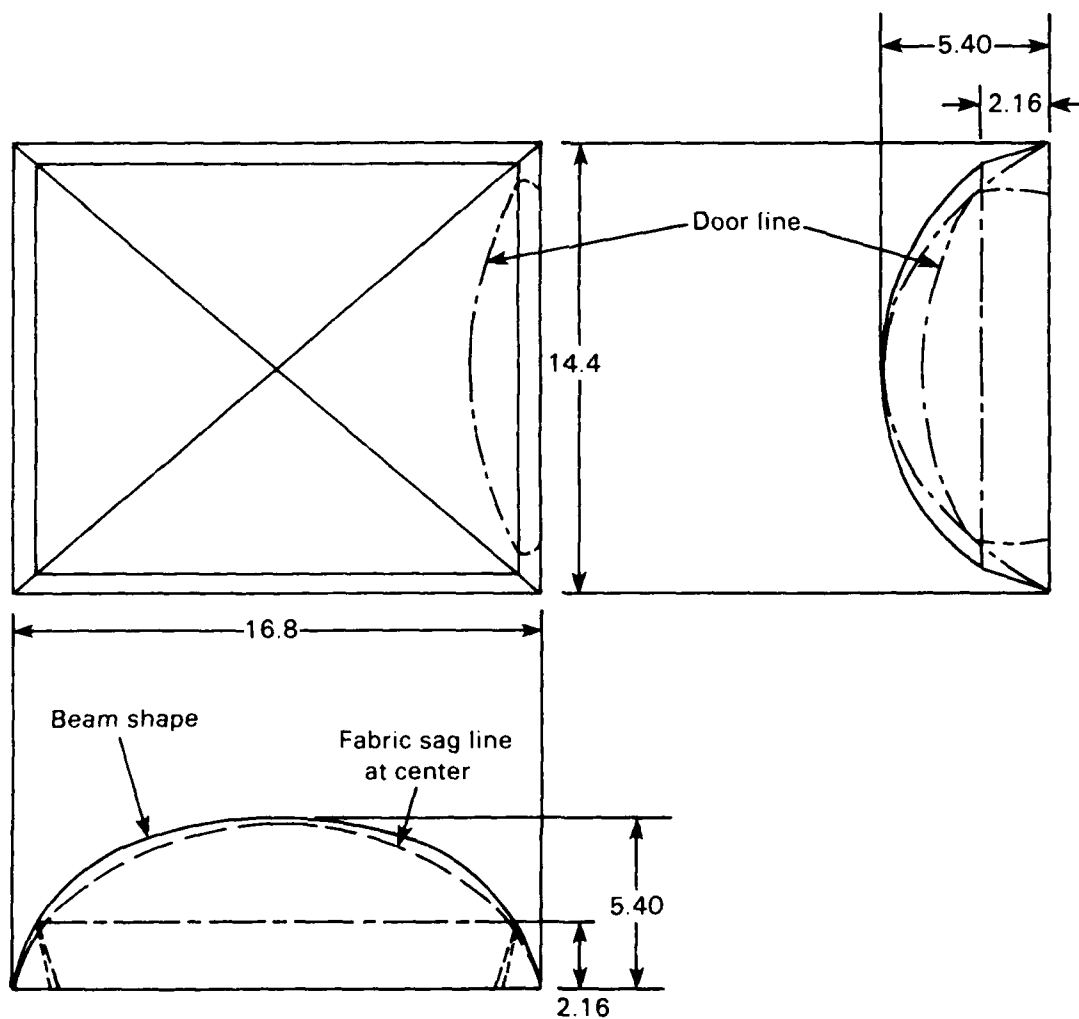
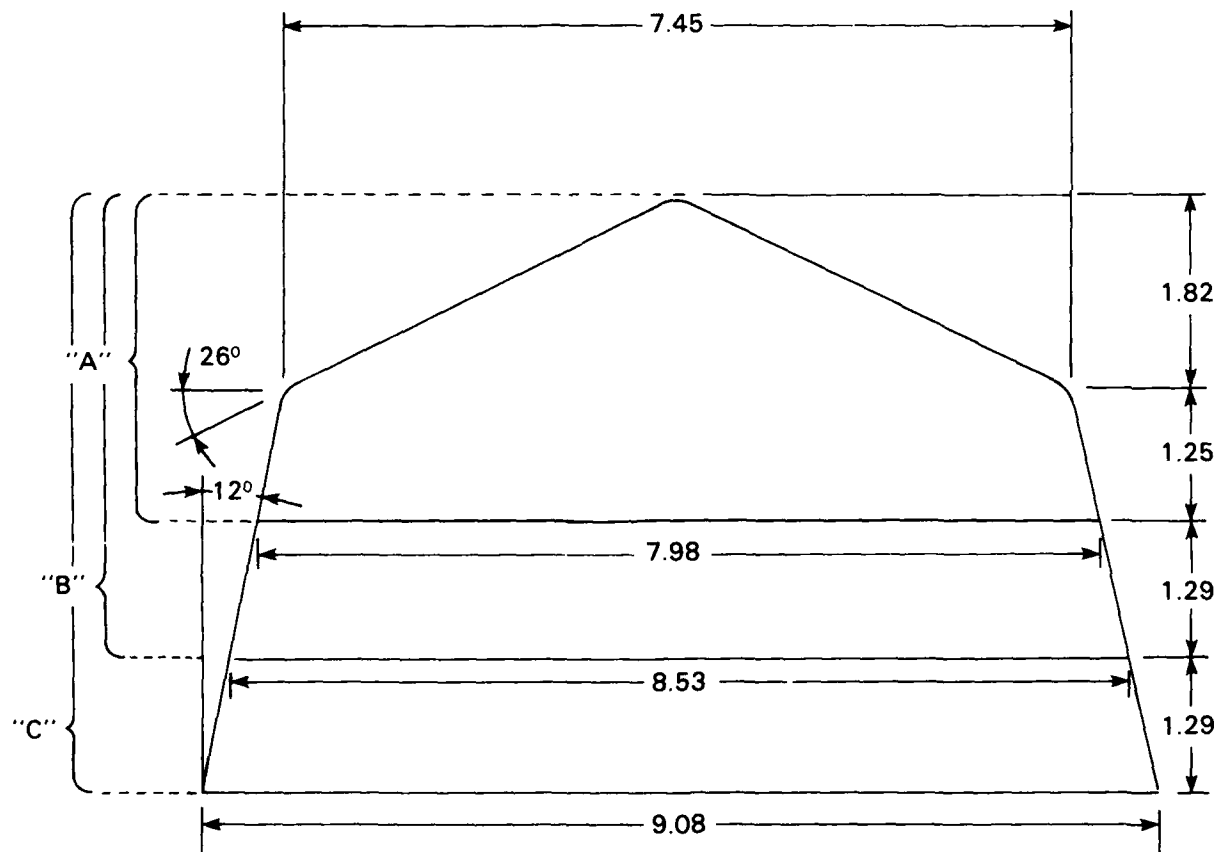


Figure 7. Dimensions of models tested at Princeton.

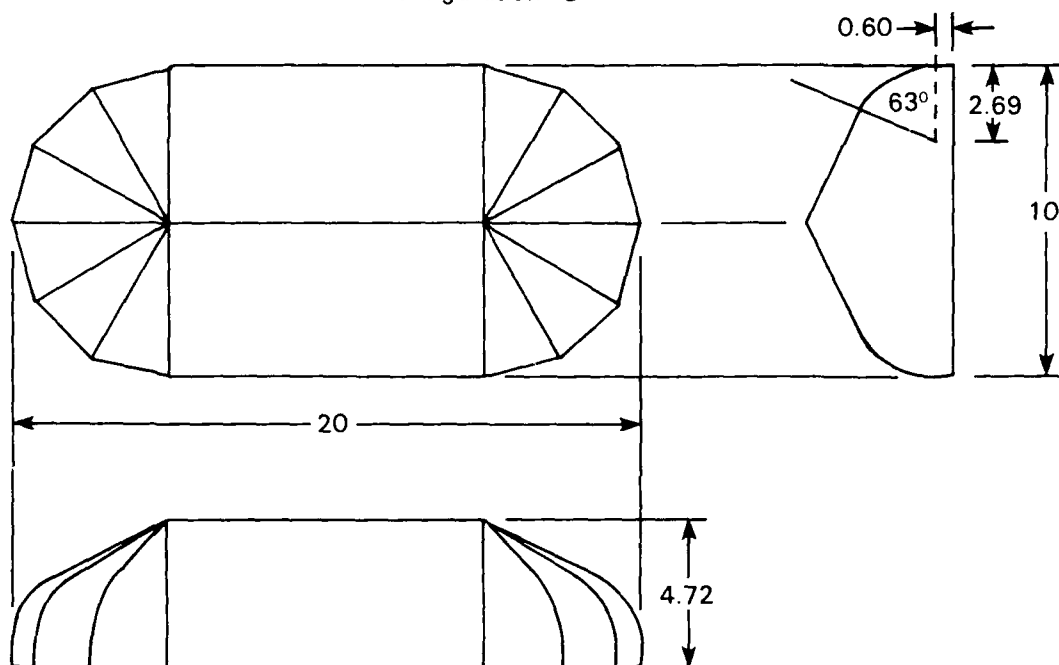


All dimensions in inches.

Figure 8. Cross-sections of model groups A, B, and C.

Dimensions of wind tunnel models tested at NCEL.

Configuration "D"



Configuration "A"

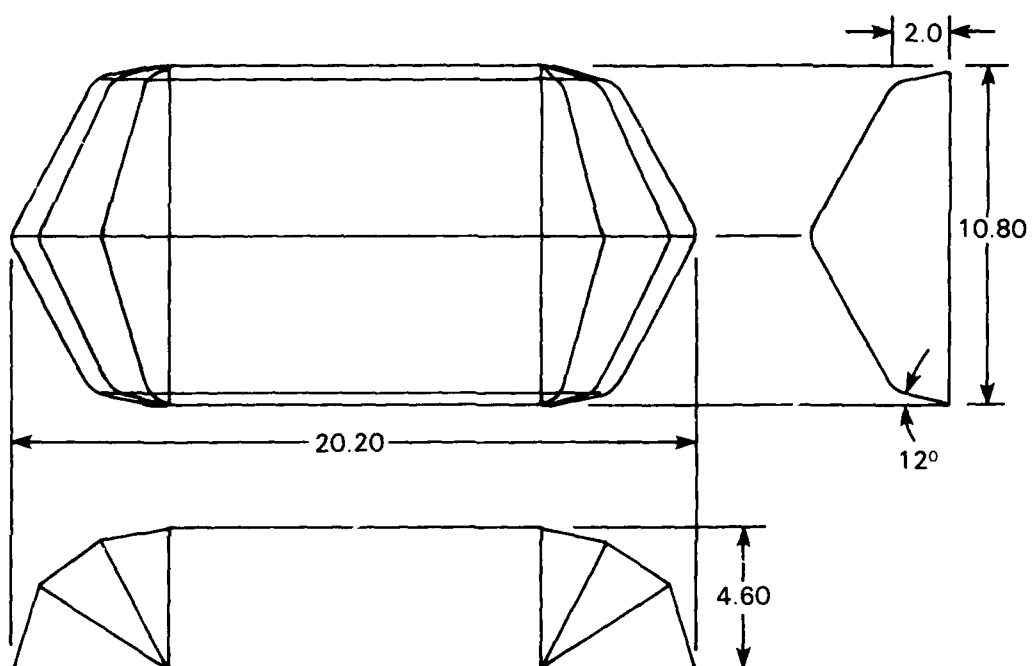
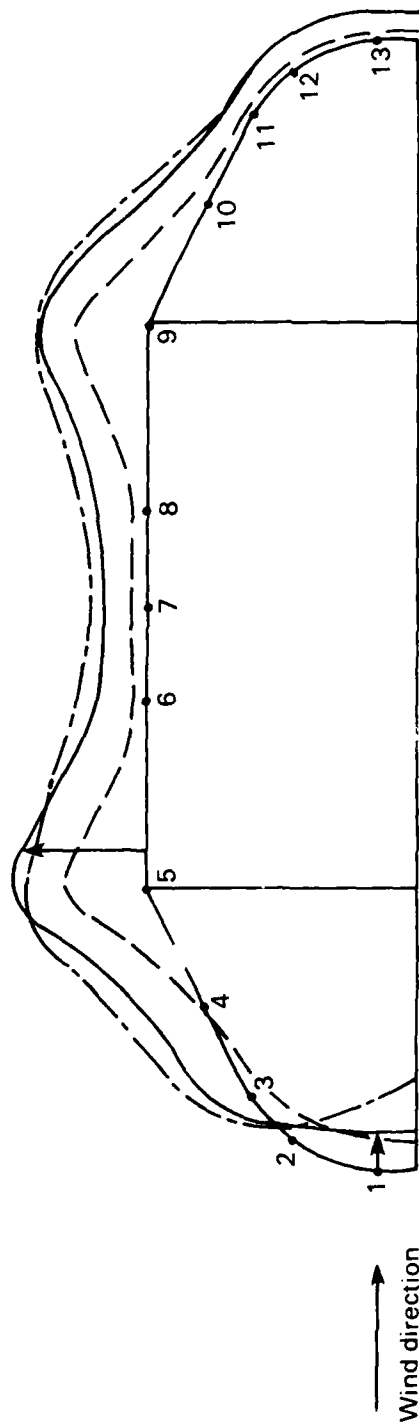


Figure 9. Models tested at NCEL.



Position	NCEL	Previous	Potential
1	0.32	0.32	0.79
2	0.20	0.38	0.24
3	-0.32	0.17	-0.41
4	-0.53	-0.03	-0.84
5	-1.43	-0.85	-1.25
6	-0.55	-0.17	-0.75
7	-0.50	-0.18	-0.60
8	-0.54	-0.18	-0.71
9	-1.12	-0.76	-1.18
10	-0.54	-0.19	-0.65
11	-0.30	-0.03	-0.24
12	-0.30	-0.03	—
13	-0.26	-0.03	—

All values refer to model centerline at 0° azimuth angle

Figure 10. Comparison of Configuration "D" wind tunnel tests.



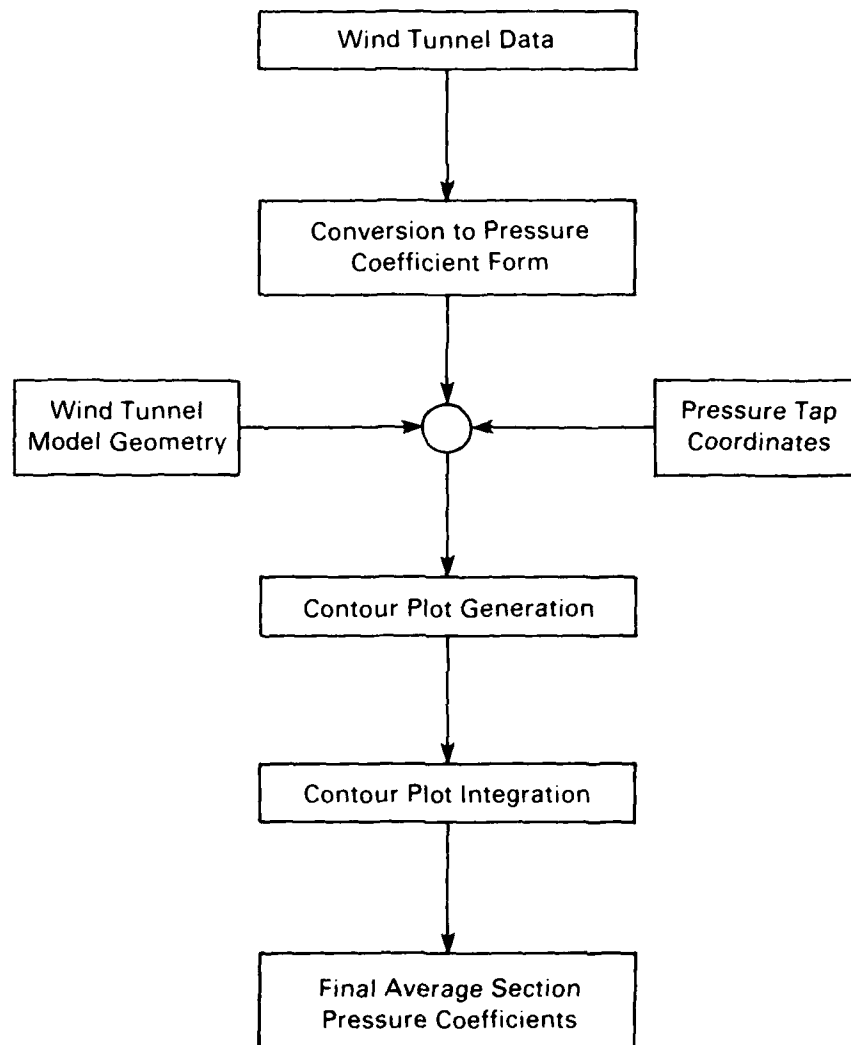
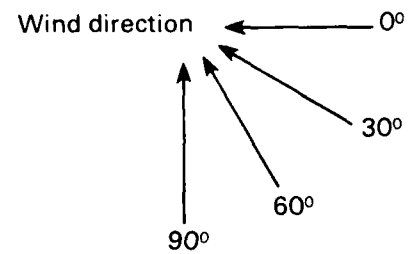
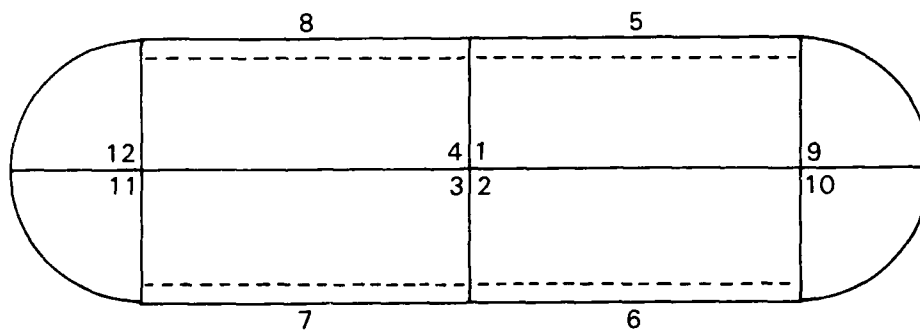


Figure 11. Data reduction and analysis flowchart.

# Section definition and wind orientation.

## Parallel-arched structures



## Diagonally-arched structures

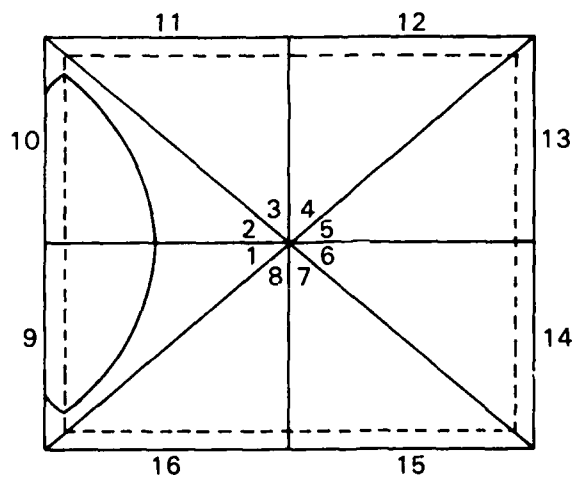
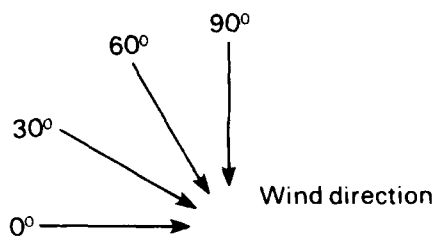
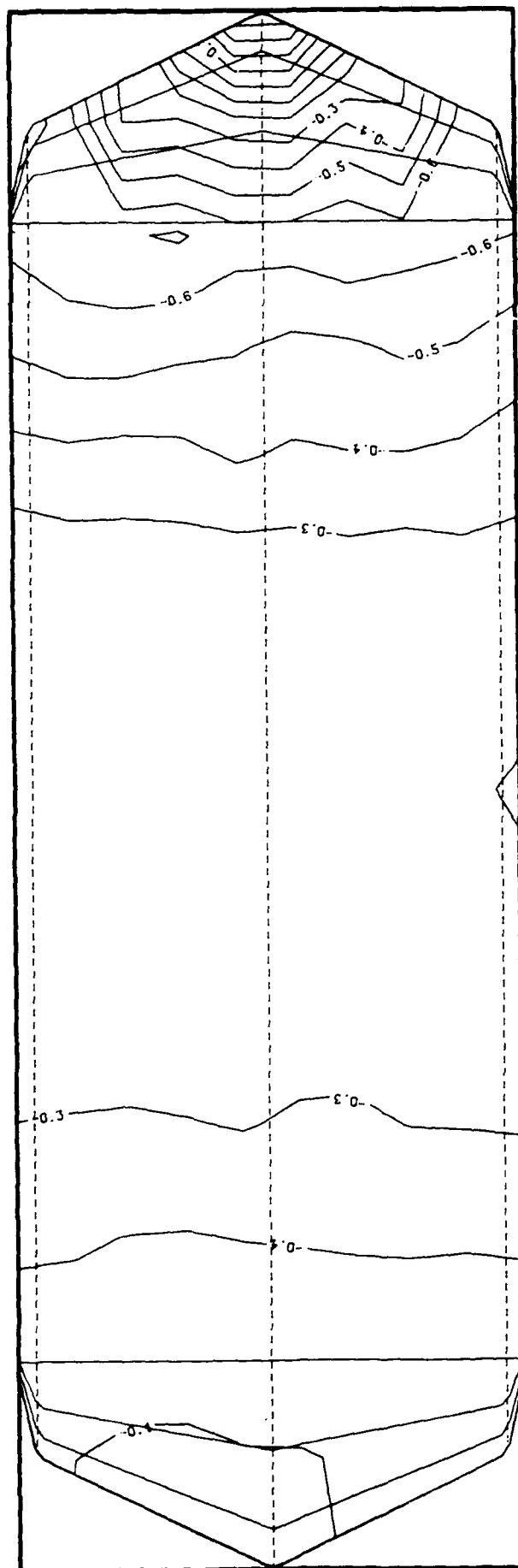


Figure 12. Structure section definitions.

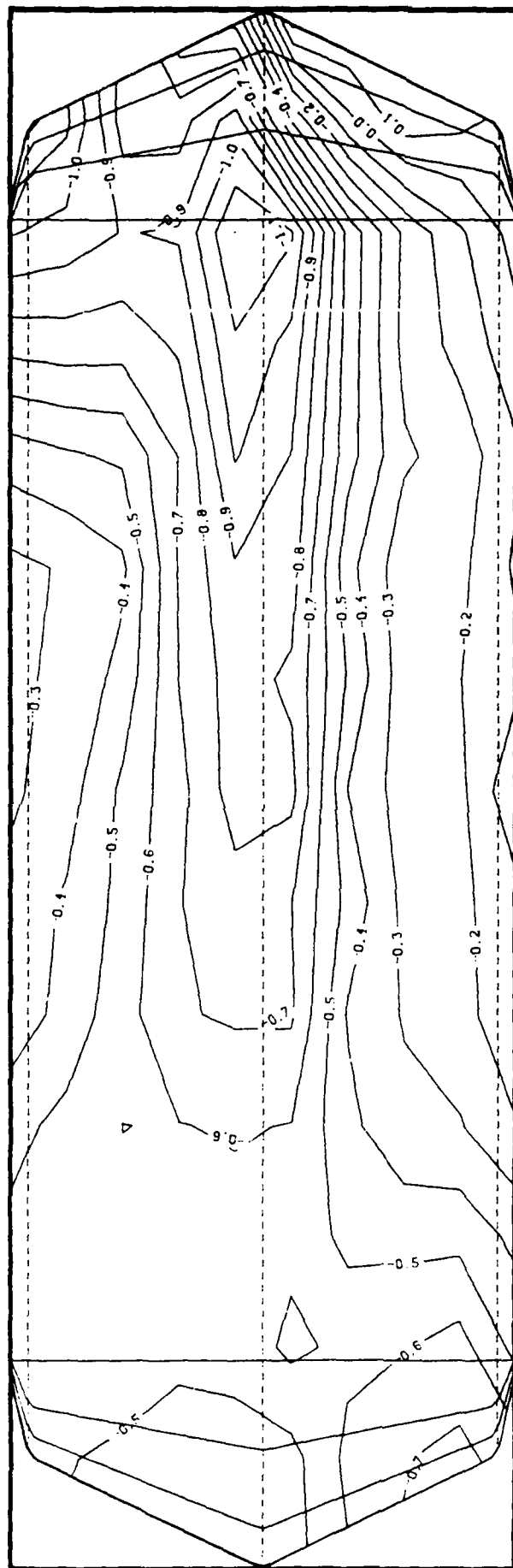
**Appendix A**  
**PRESSURE CONTOUR PLOTS**

C<sub>p</sub> CONTOURS, AZIMUTH ANGLE=0



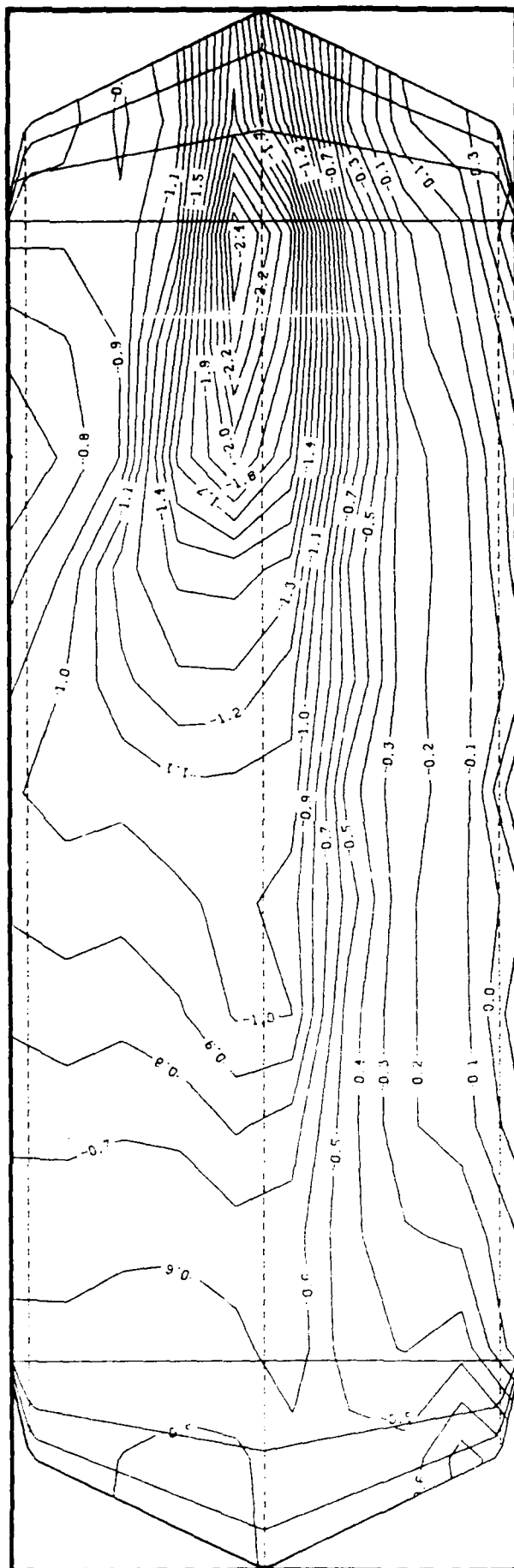
100 AZIMUTH

C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-30



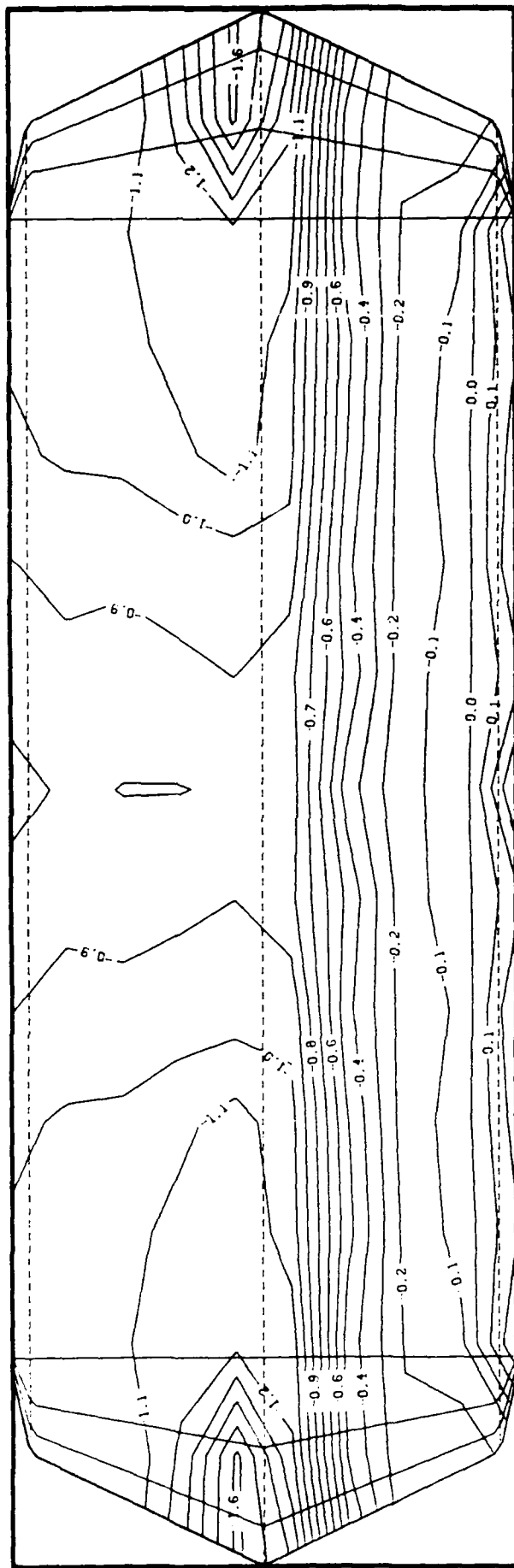
!80 AZIMUTH

Cp CONTOURS, AZIMUTH ANGLE-60



00 9214734

C<sub>p</sub> CONTOURS, AZIMUTH ANGLE -90

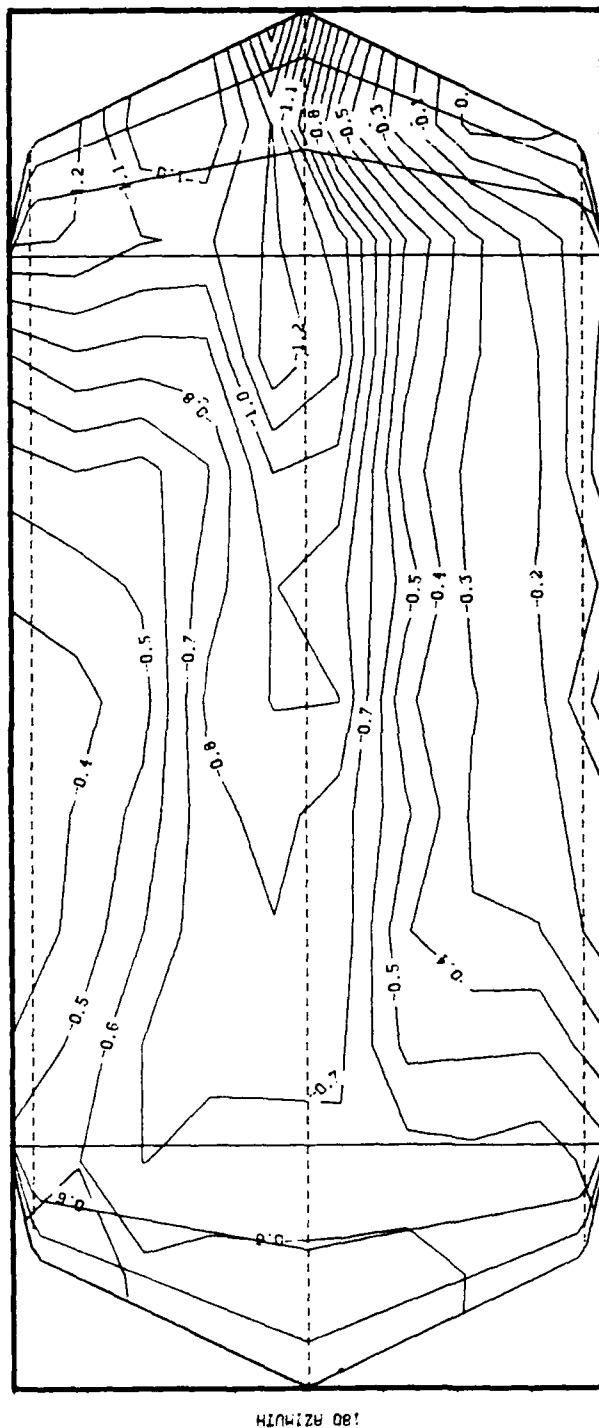


1.80 AZIMUTH

A-6

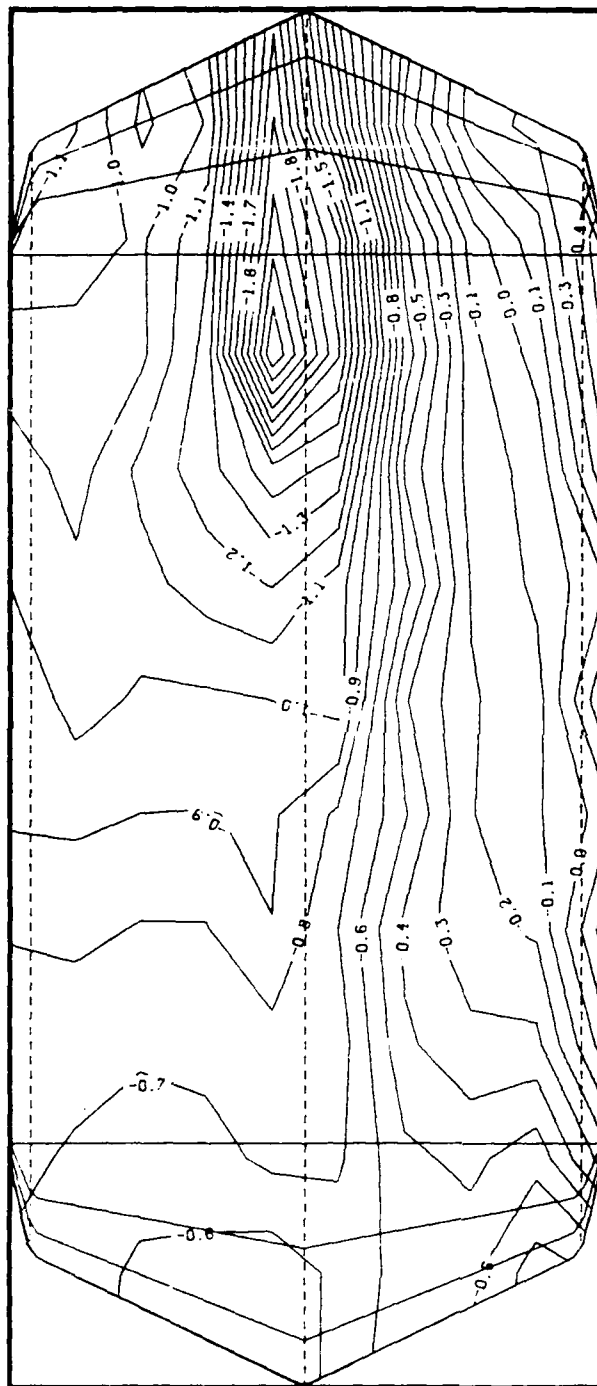


C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-30



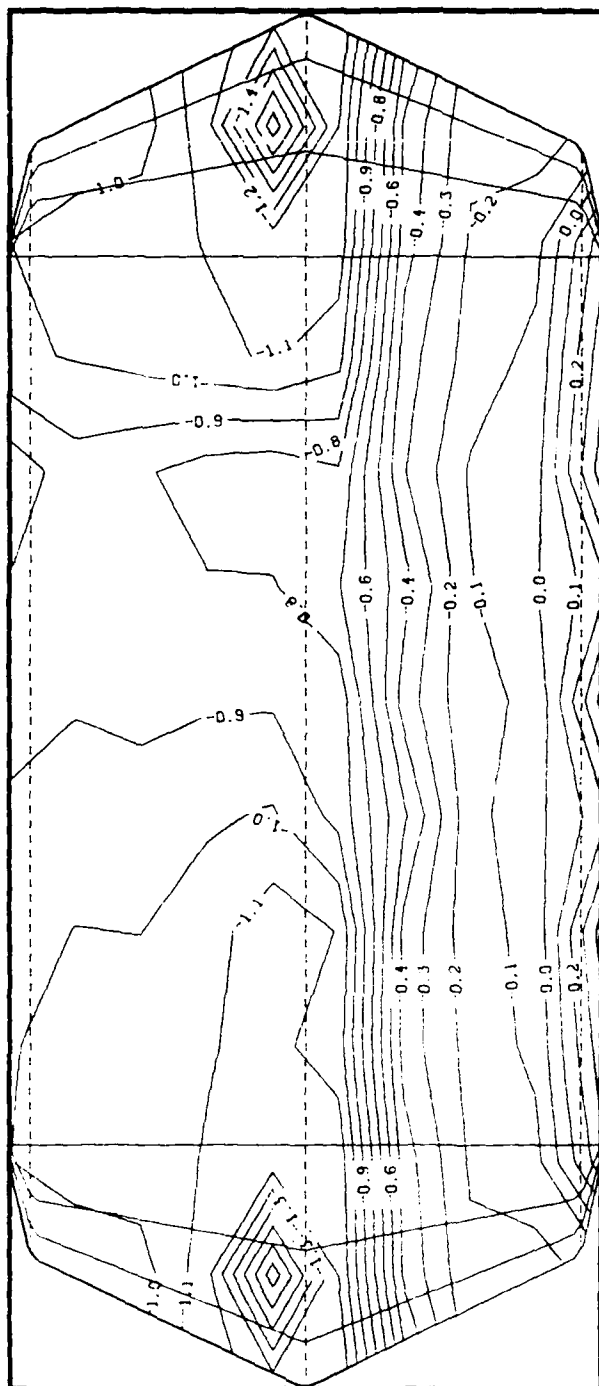
180 AZIMUTH

C<sub>p</sub> CONTOURS, AZIMUTH ANGLE -60



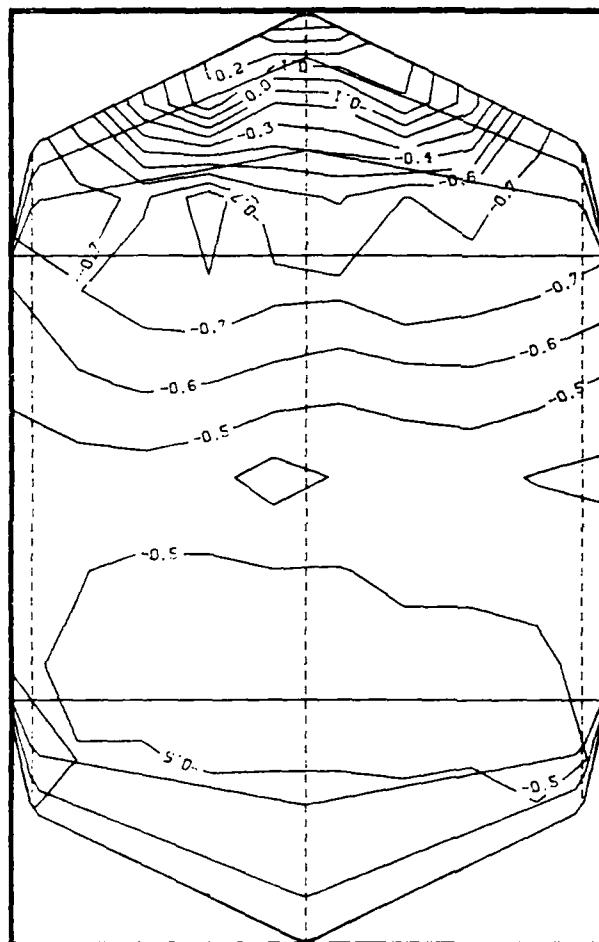
1.00 AZIMUTH

Cp CONTOURS, AZIMUTH ANGLE-90



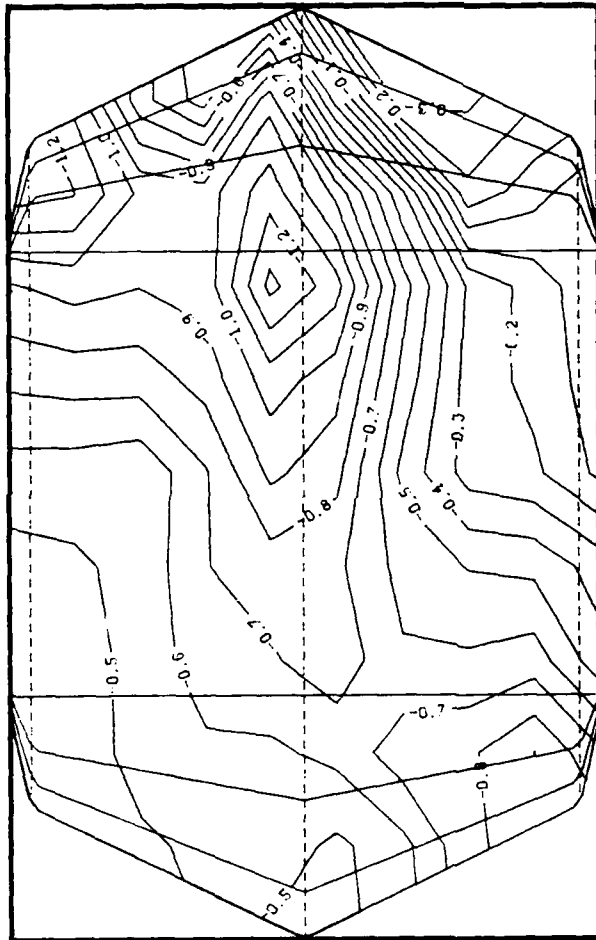
AZIMUTH

C<sub>p</sub> CONTOURS, AZIMUTH ANGLE=0



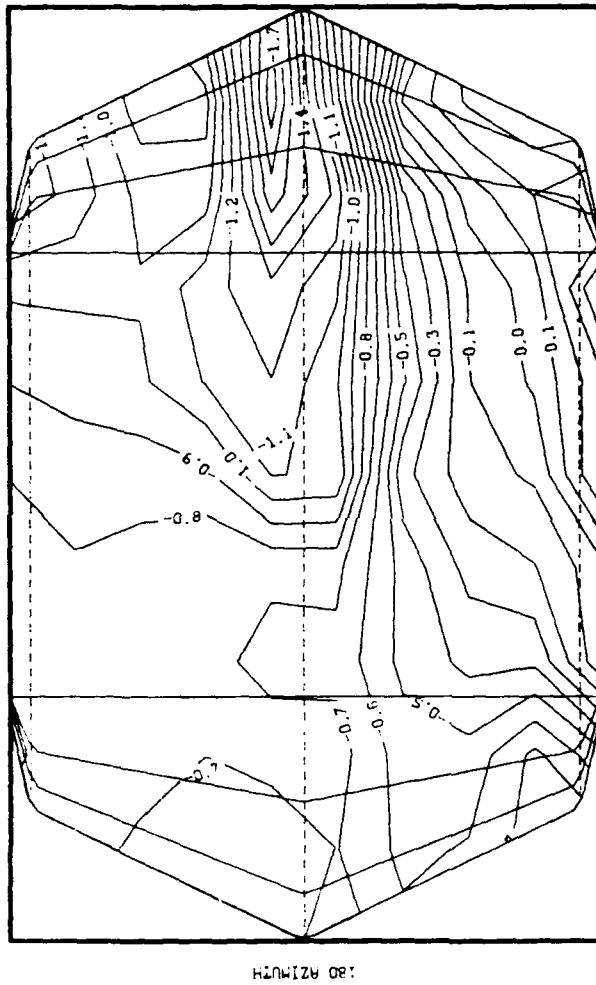
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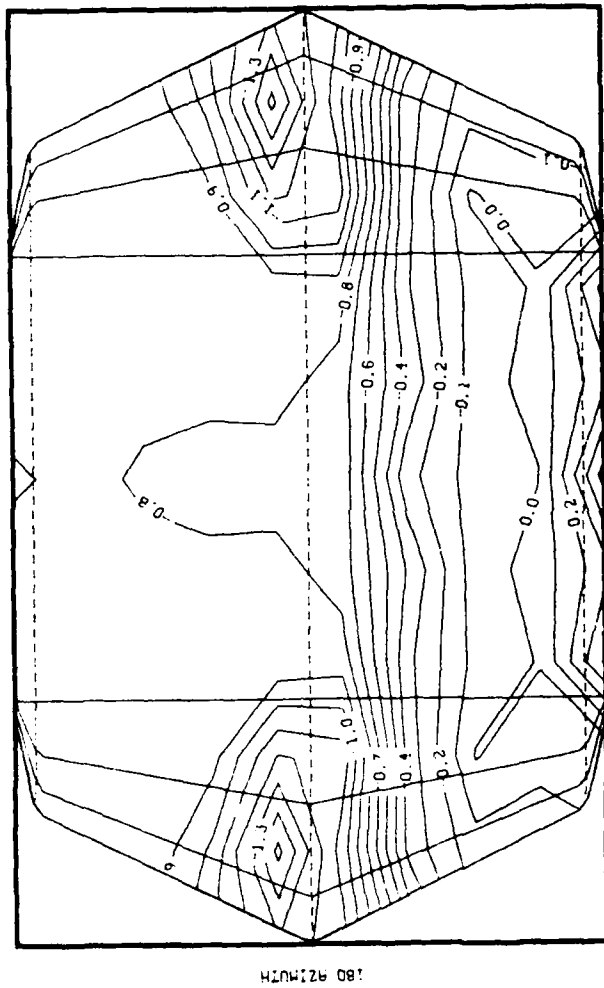


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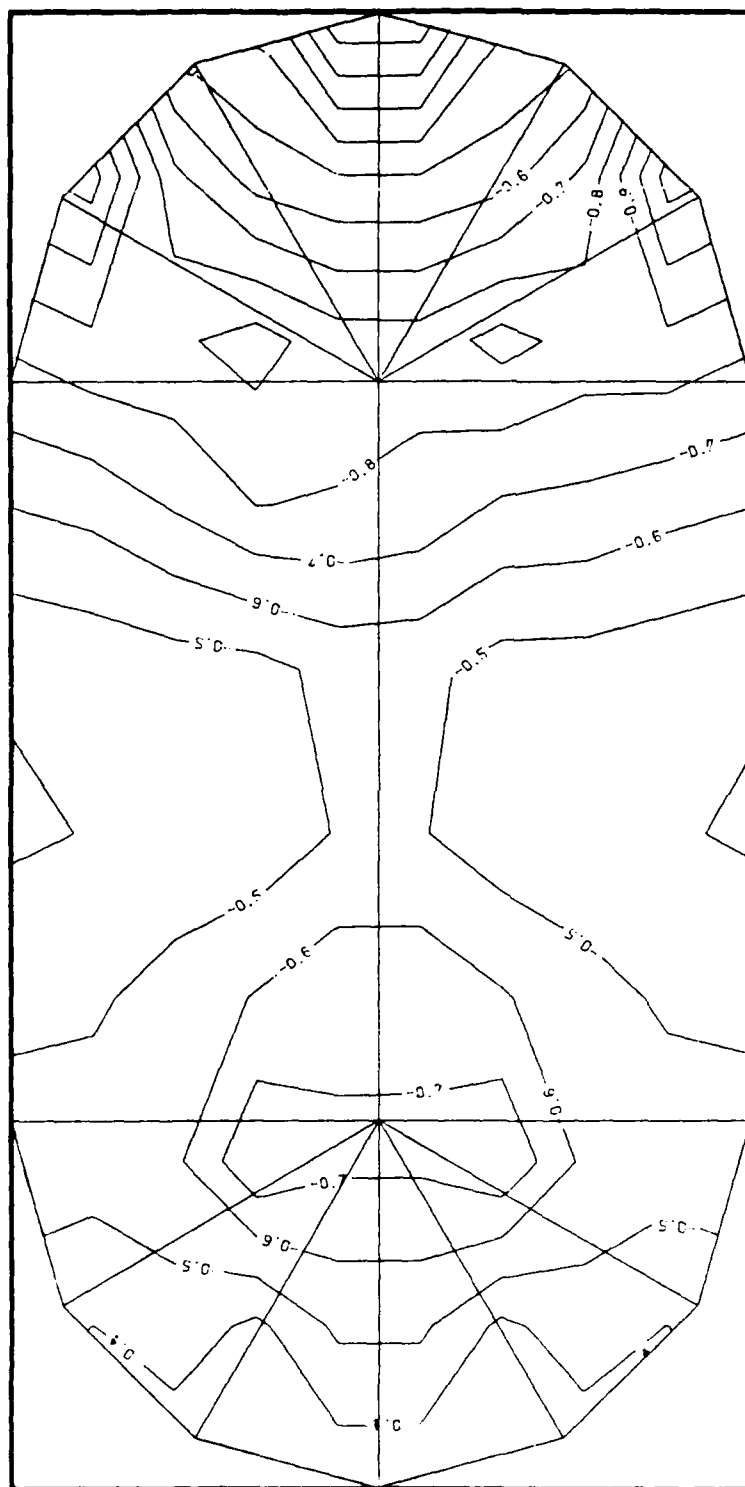
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C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-90



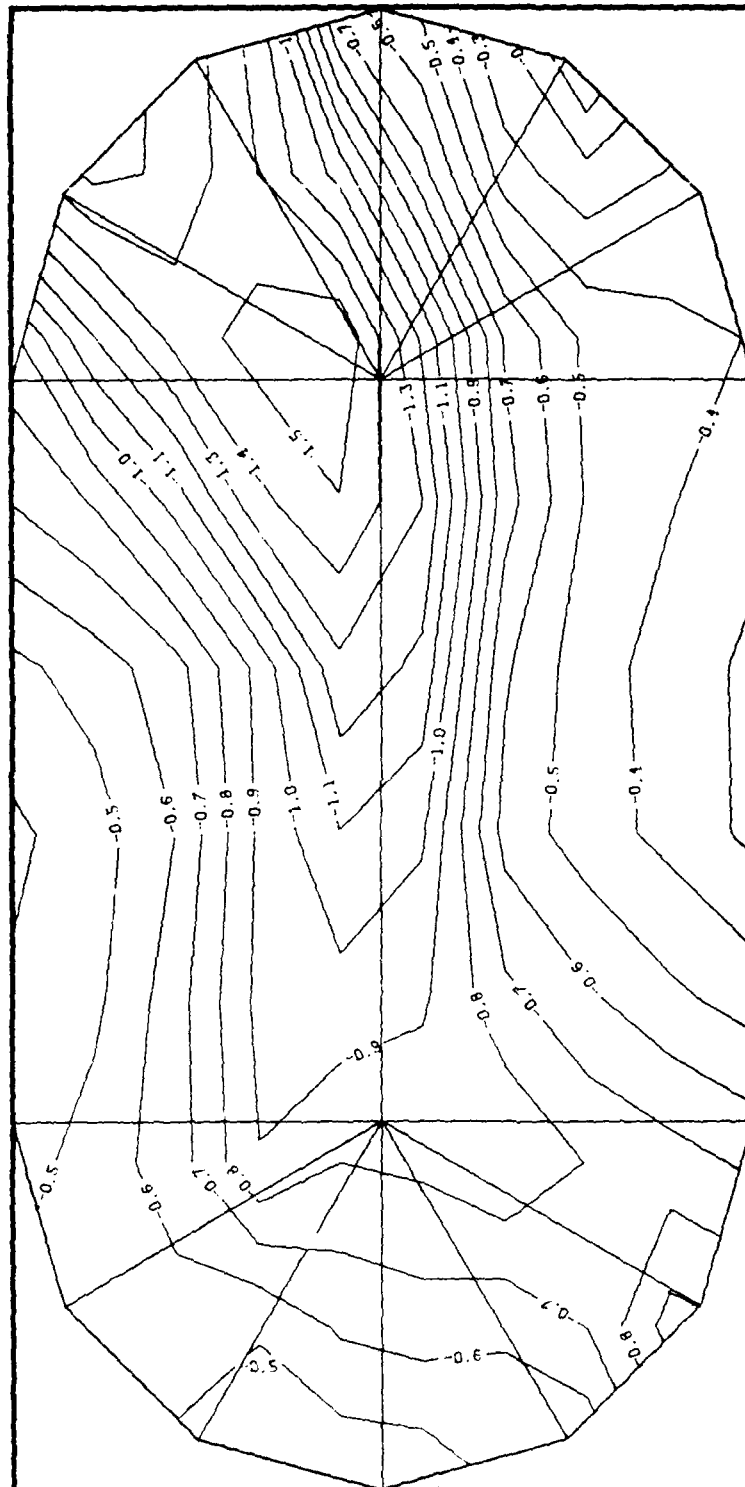
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WINDWIZ CP1

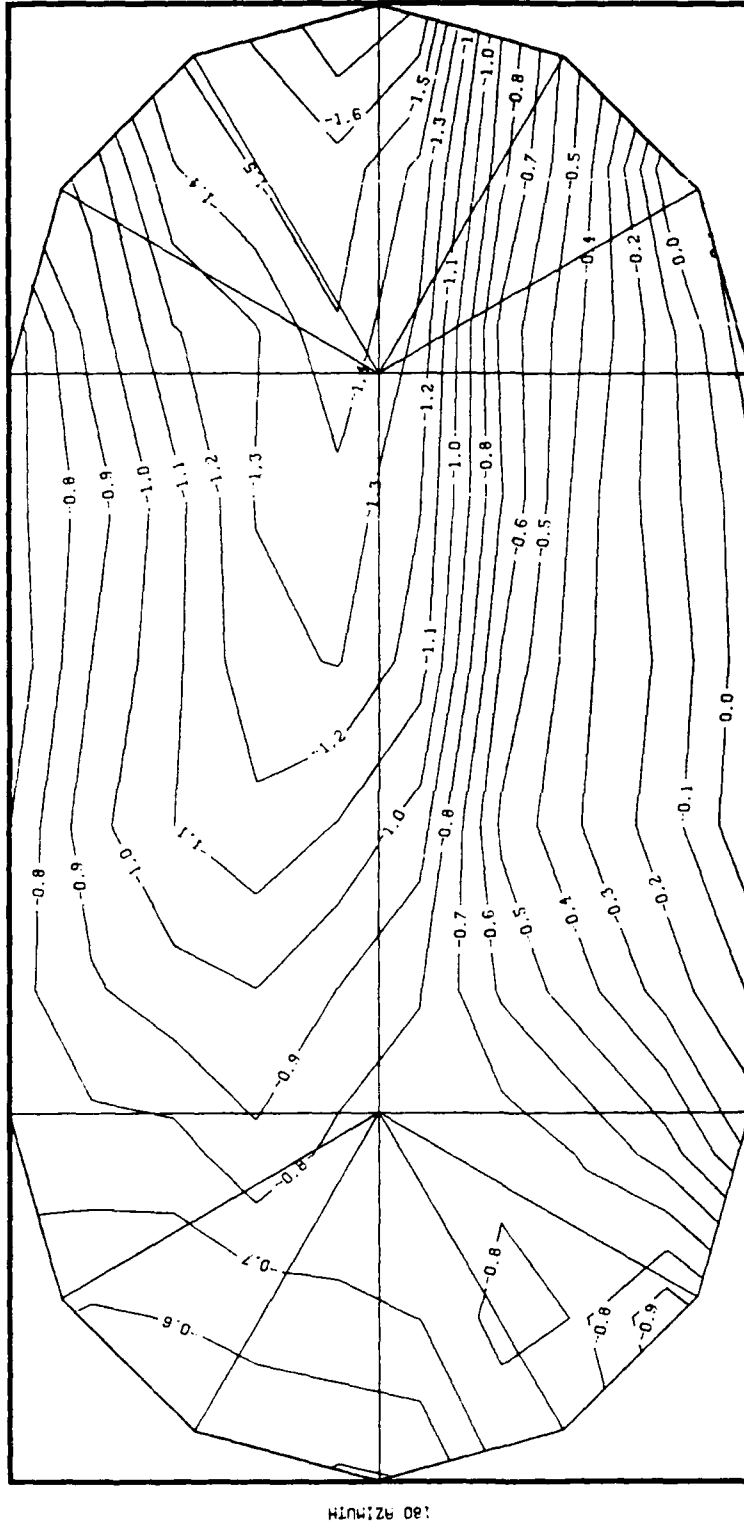


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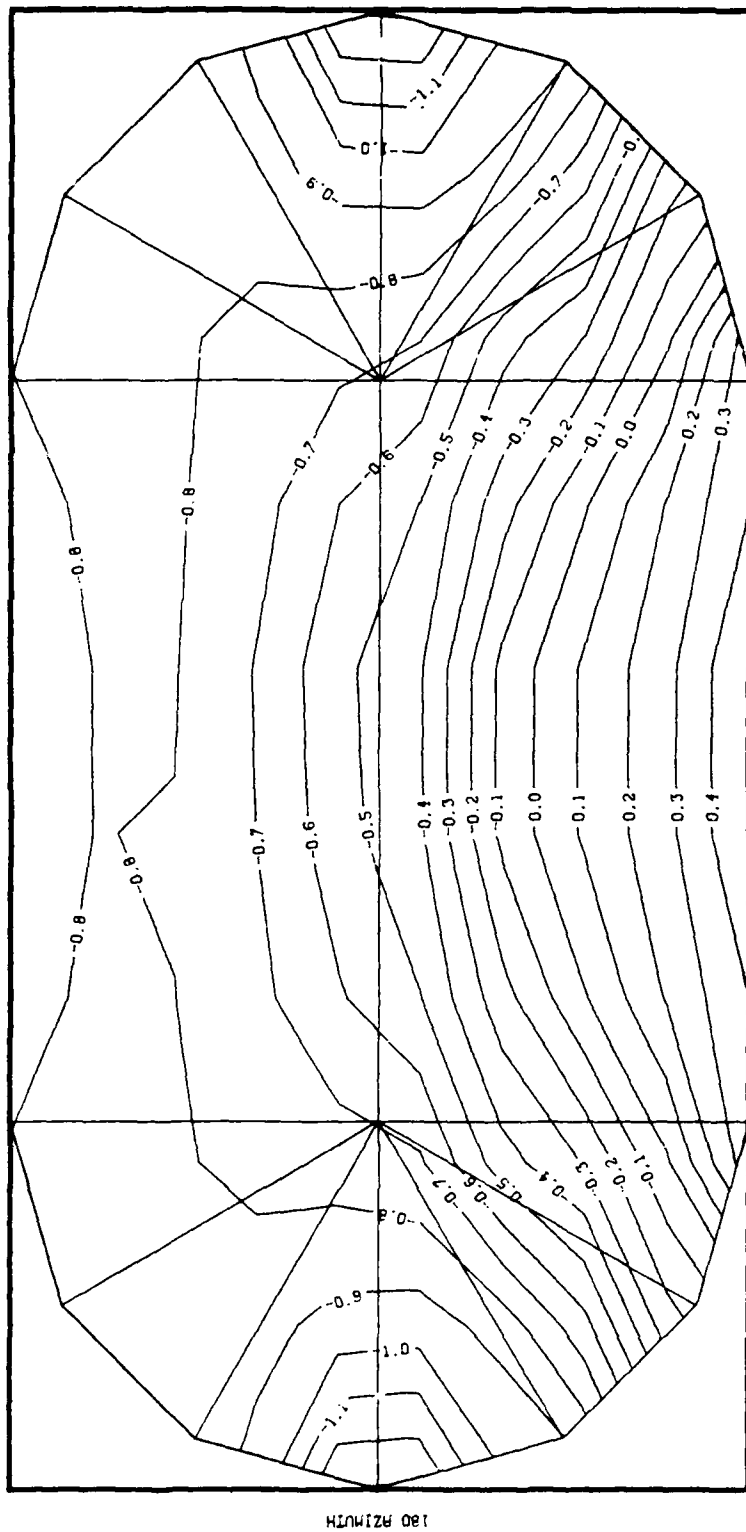


180 AZIMUTH

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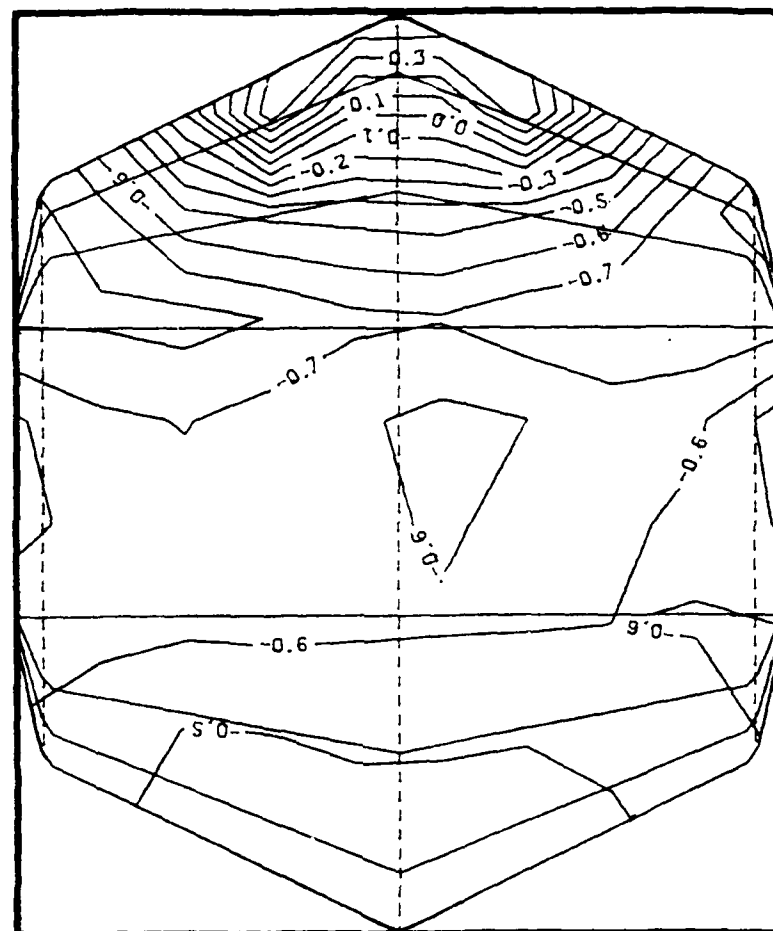


C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-90



180 AZIMUTH

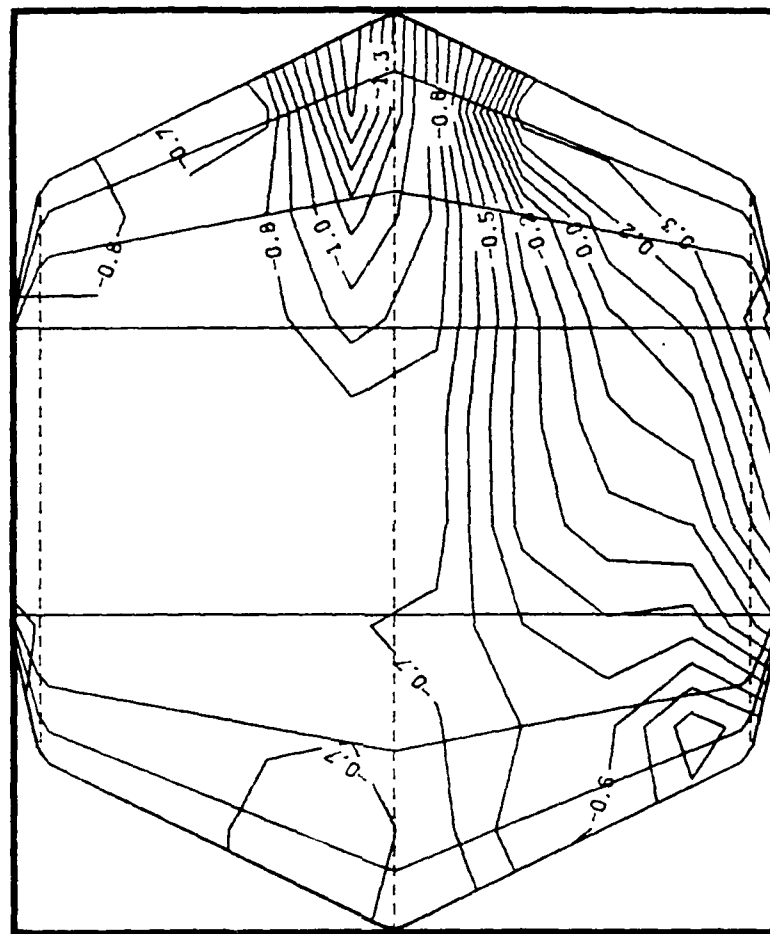
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180 AZIMUTH

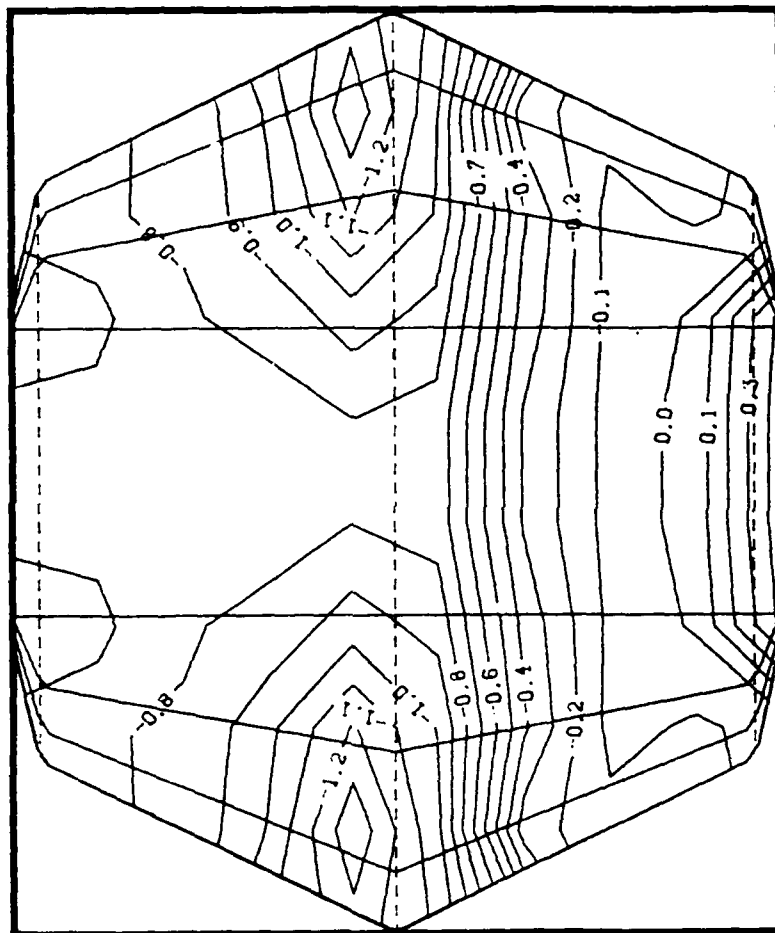
A-19

C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-60



180 AZIMUTH

Cp CONTOURS, AZIMUTH ANGLE=90

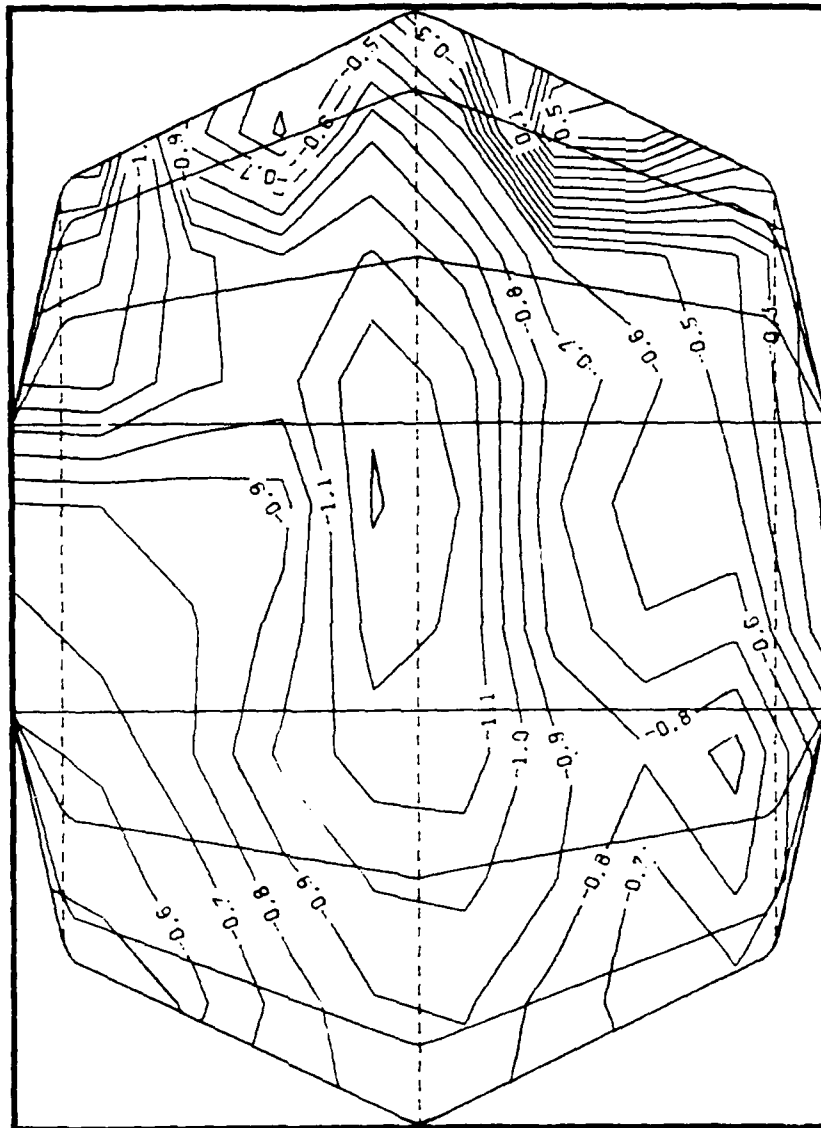


180 AZIMUTH

A-22

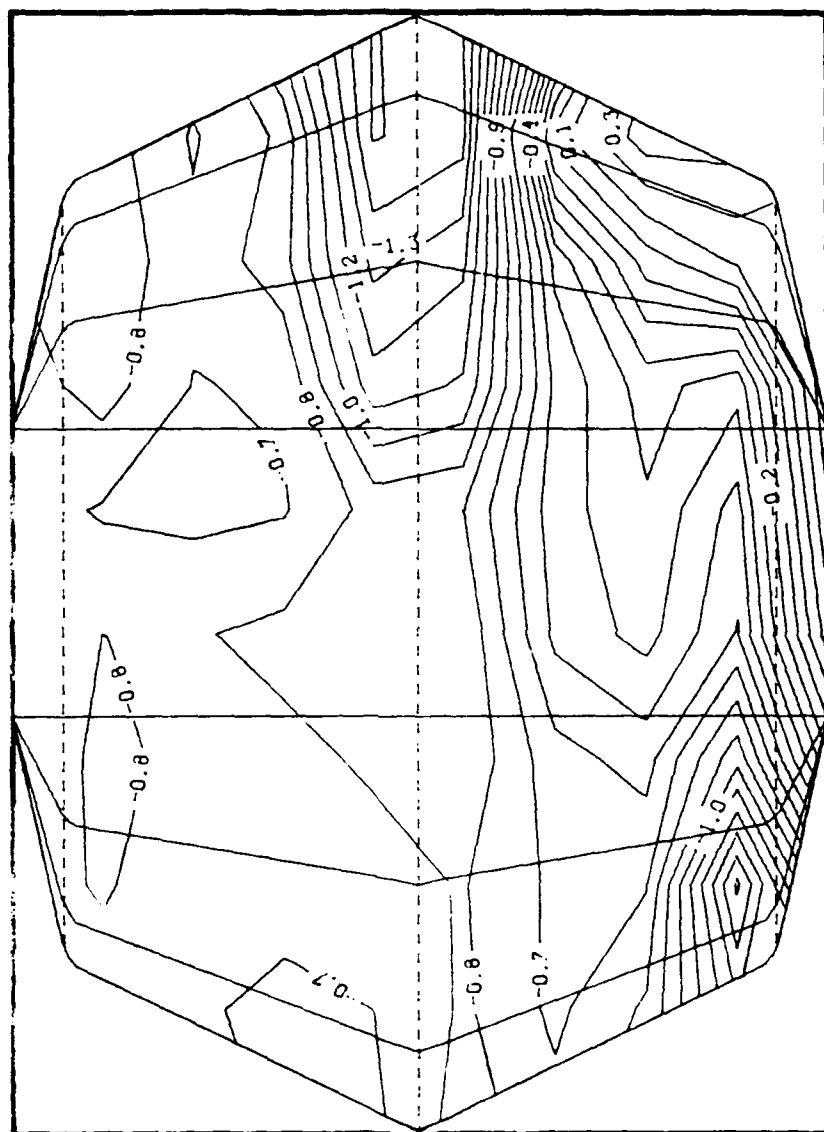


C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-30

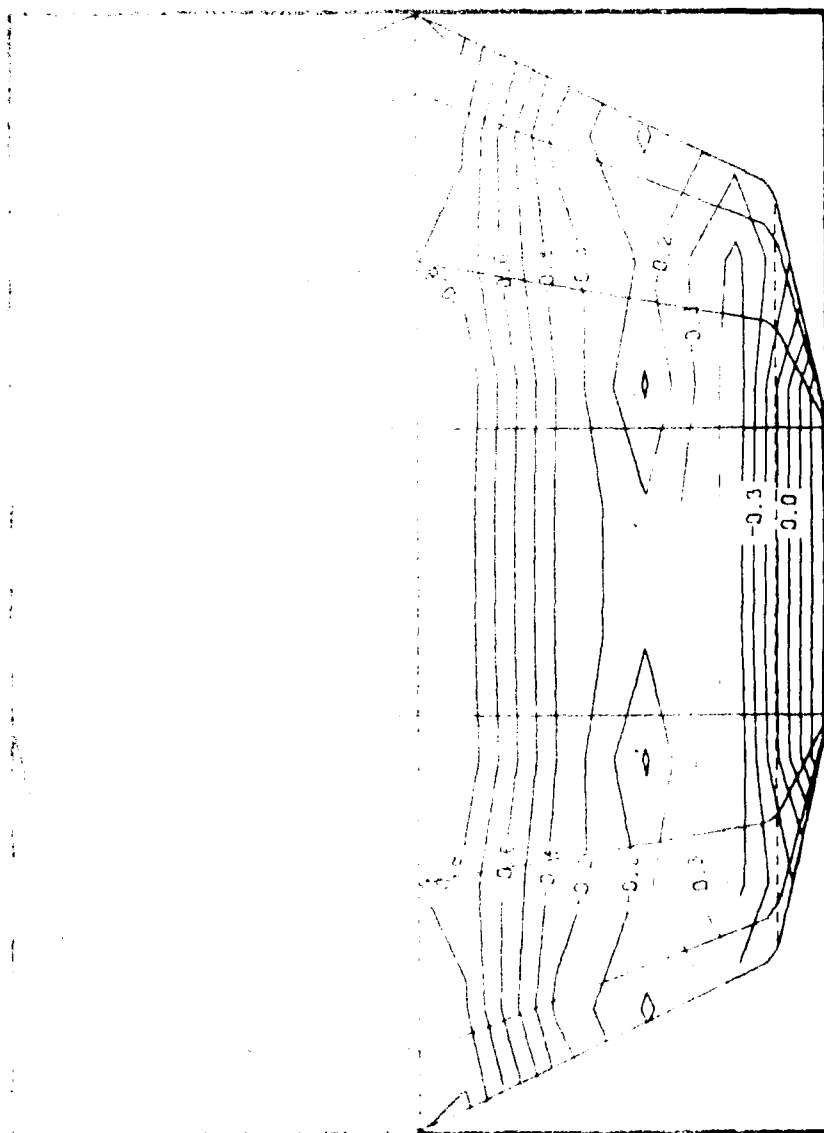


180 AZIMUTH

# Cp CONTOURS, AZIMUTH ANGLE=60



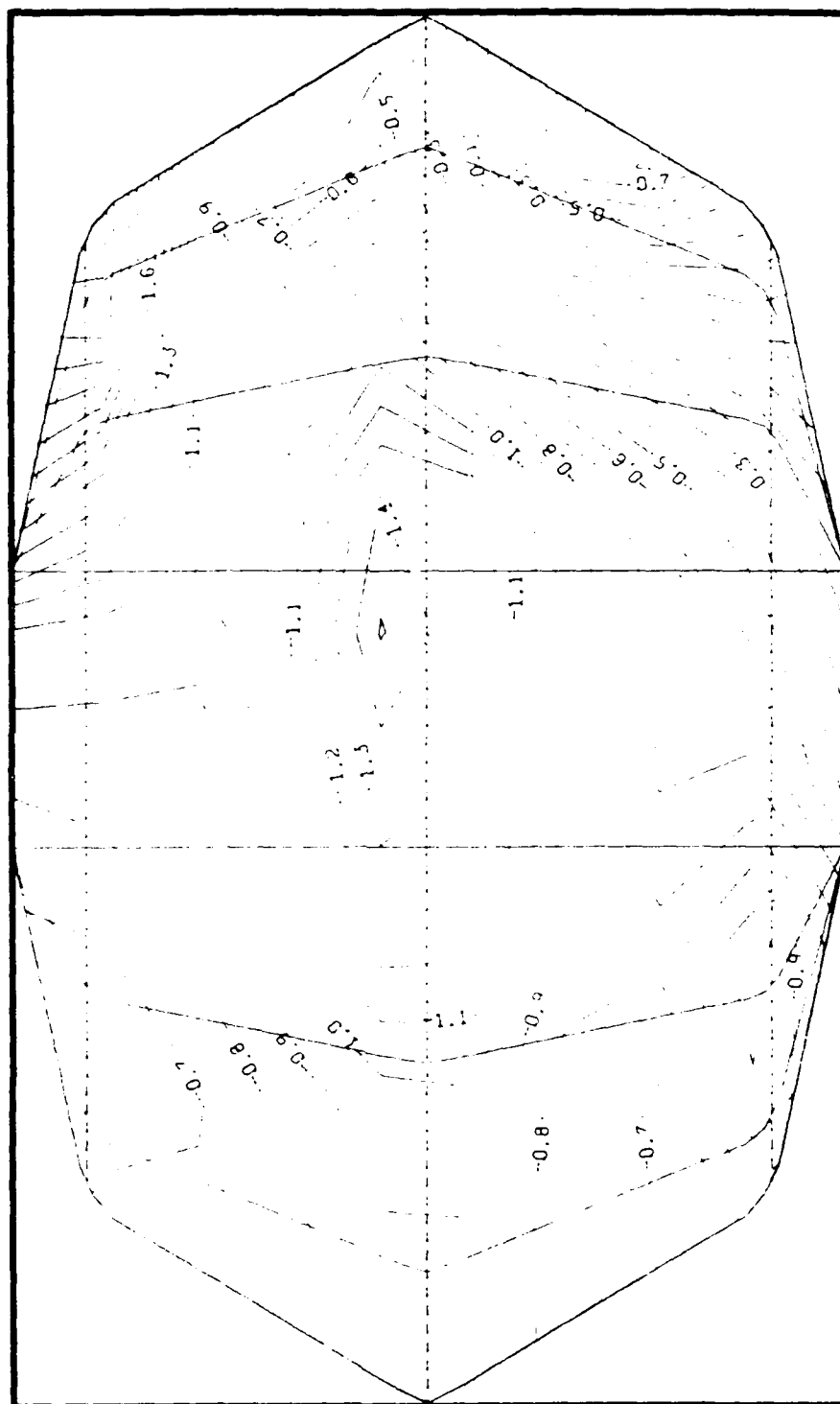
180 AZIMUTH



180 INCHES



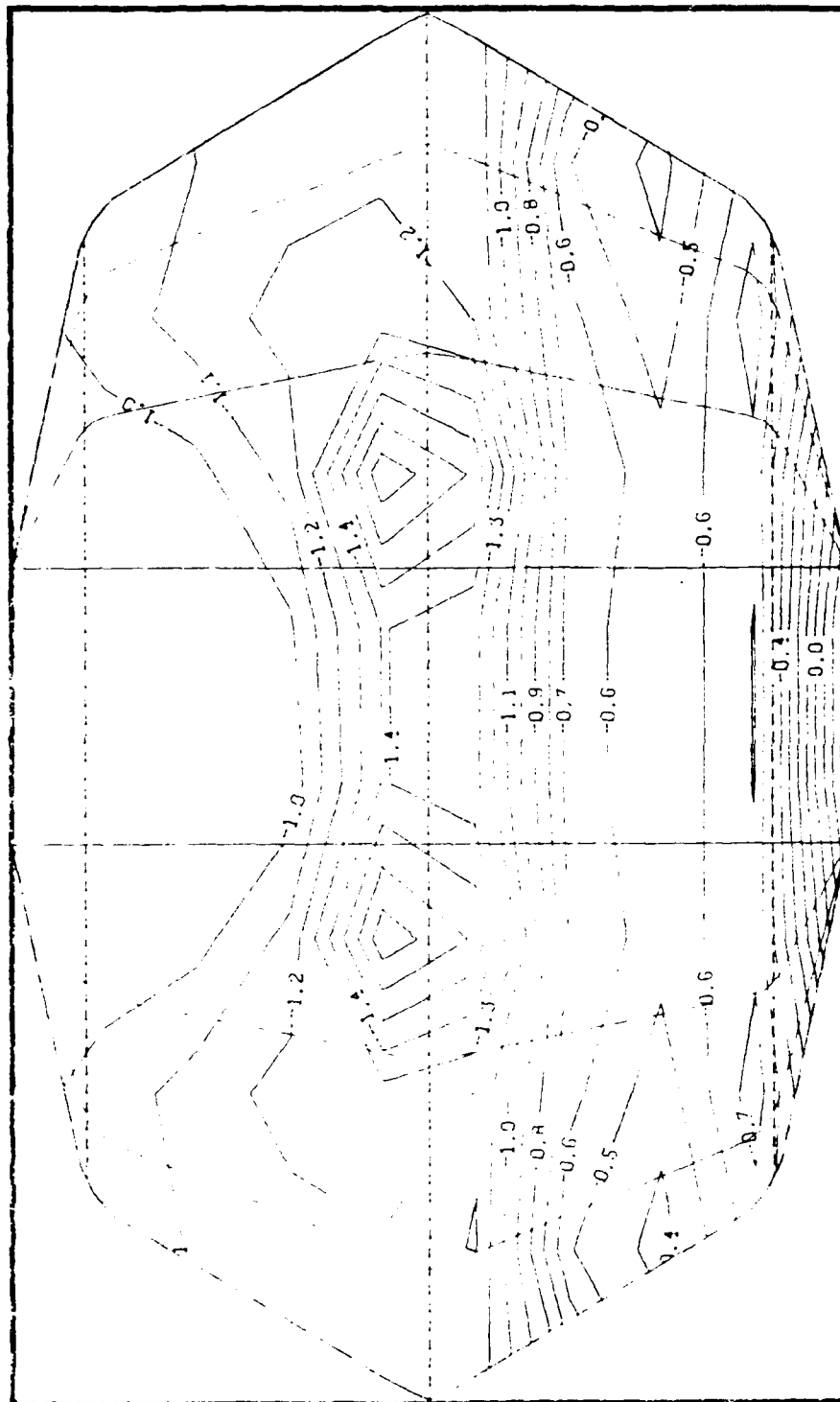
C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-30



180 AZIMUTH

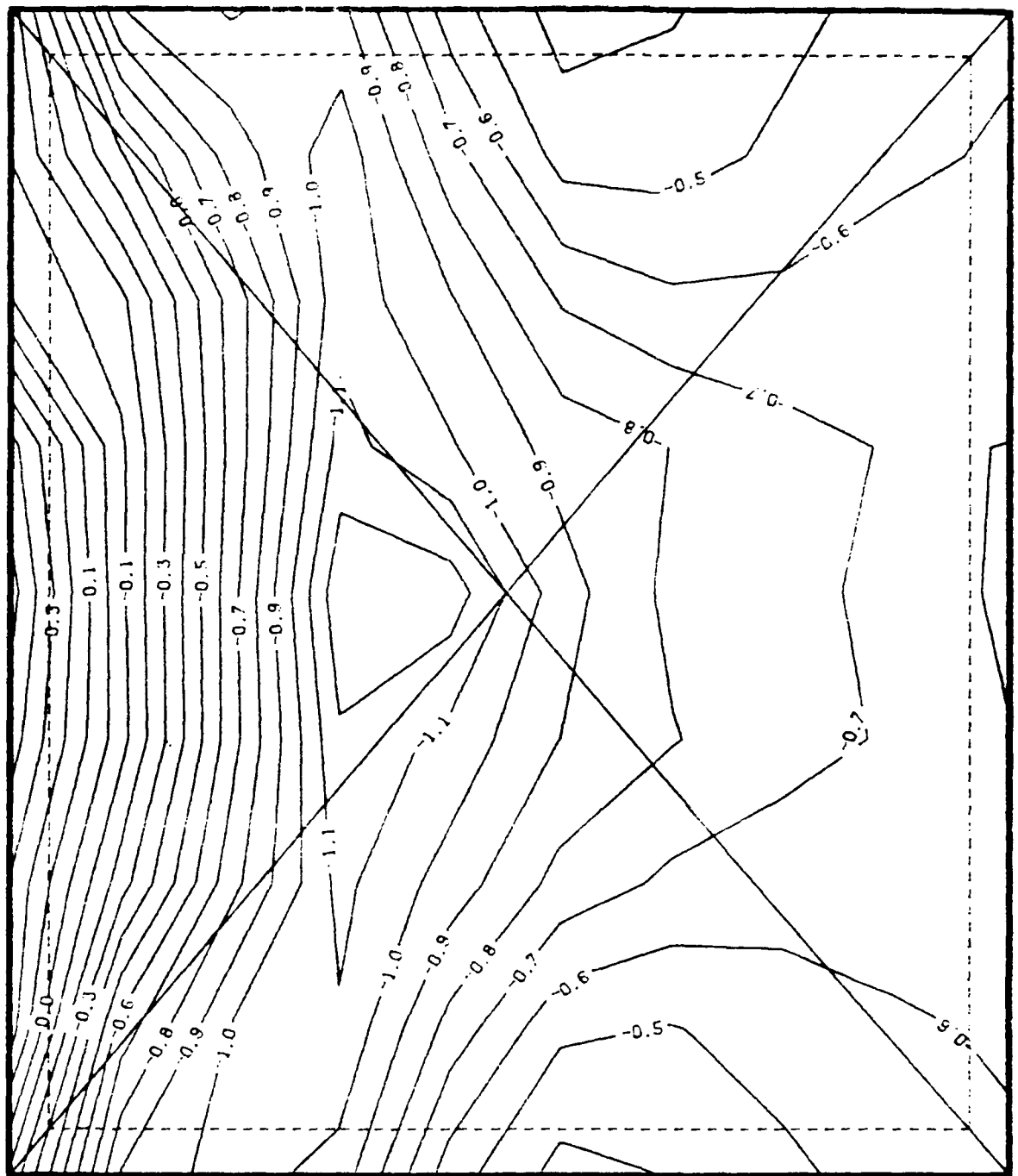
A-28

# C<sub>p</sub> CONTOURS, AZIMUTH ANGLE=90



180-100-081

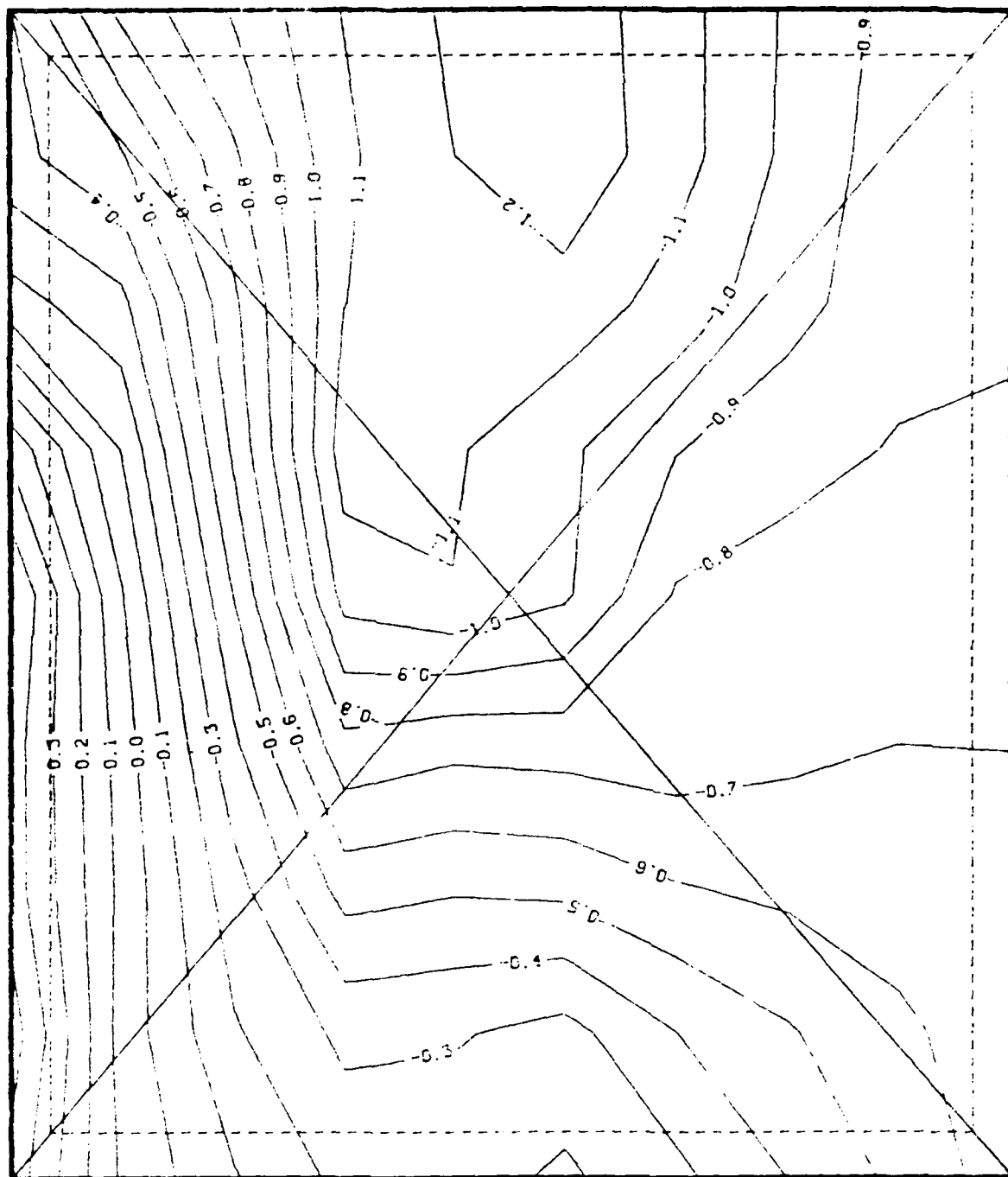
Cp CONTOURS, AZIMUTH ANGLE-90



0000 8000

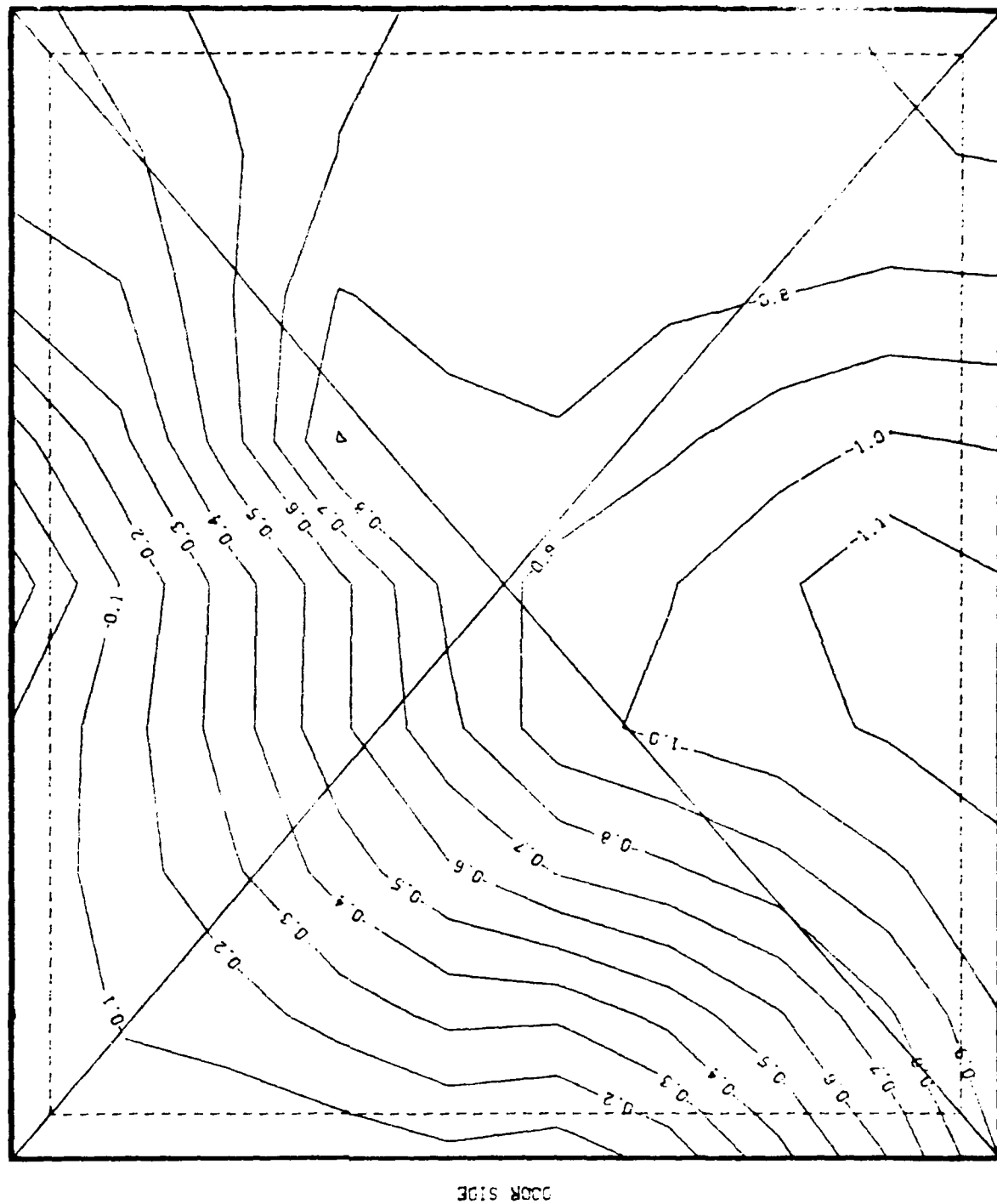


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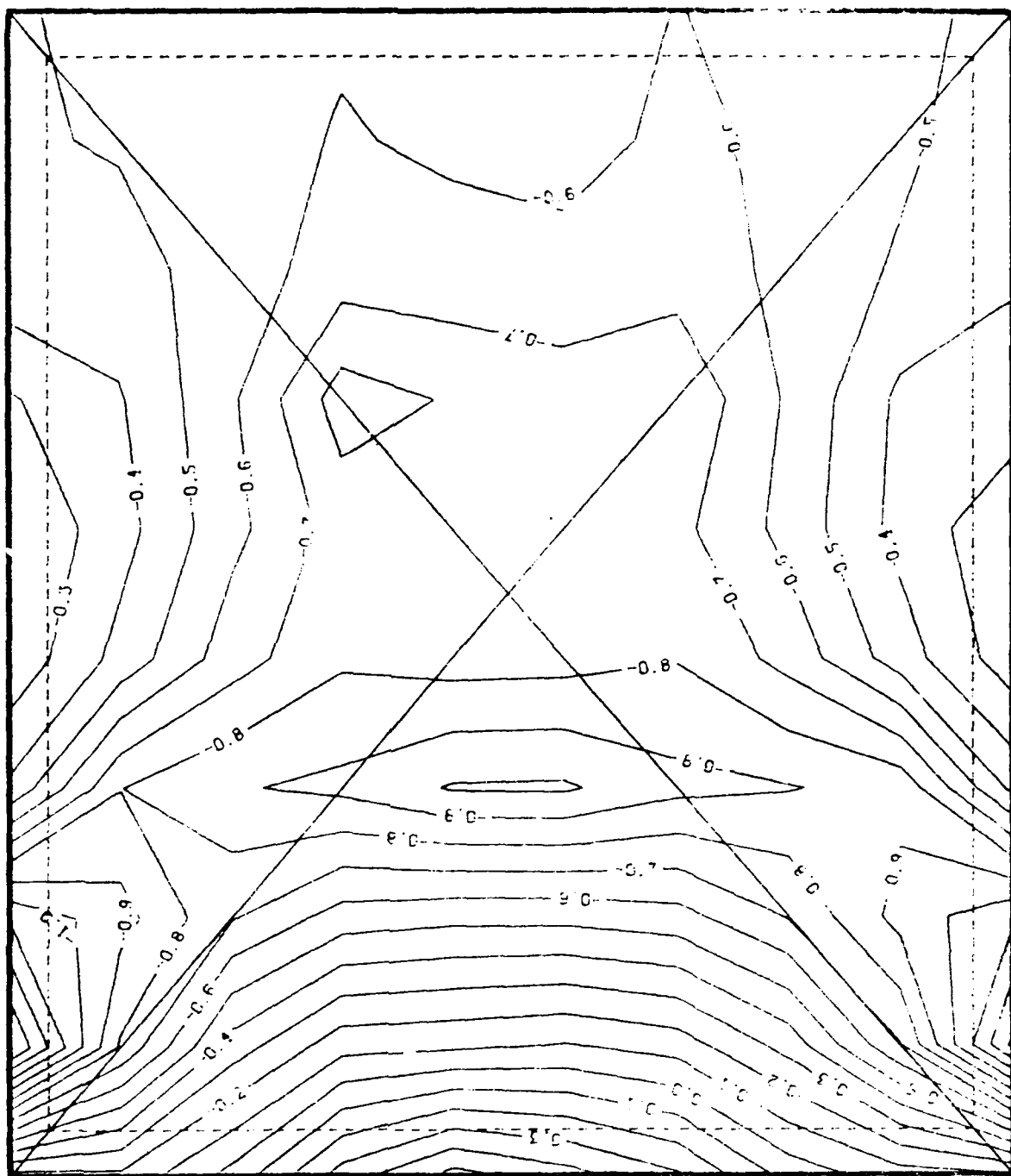


DOOR SIDE

# C<sub>p</sub> CONTOURS, AZIMUTH ANGLE-30



Cp CONTOURS, AZIMUTH ANGLE=0



DOOR SIDE

**Appendix B**  
**SECTION AVERAGE PRESSURE COEFFICIENTS**

Recommended Section Pressure Coefficients, Model A1

Section	Azimuth Angle			
	0	30	60	90
1	-0.40	-0.70	-1.30	-1.00
2	-0.40	-0.45	-0.65	-0.40
3	-0.35	-0.45	-0.45	-0.40
4	-0.35	-0.60	-0.85	-1.00
5	-0.40	-0.50	-0.80	-1.00
6	-0.40	-0.05	0.40	0.50
7	-0.35	-0.25	0.25	0.50
8	-0.35	-0.40	-0.80	-1.00
9	-0.35	-0.85	-1.30	-1.25
10	-0.35	-0.10	-0.30	-0.65
11	-0.45	-0.65	-0.60	-0.65
12	-0.45	-0.50	-0.55	-1.25

Recommended Section Pressure Coefficients, Model A3

Section	Azimuth Angle			
	0	30	60	90
1	-0.45	-0.80	-1.20	-1.00
2	-0.45	-0.50	-0.50	-0.40
3	-0.35	-0.50	-0.40	-0.40
4	-0.35	-0.65	-0.85	-1.00
5	-0.45	-0.60	-1.05	-0.95
6	-0.45	-0.10	0.50	0.55
7	-0.35	-0.30	0.25	0.55
8	-0.35	-0.45	-0.85	-0.95
9	-0.35	-0.95	-1.25	-1.15
10	-0.35	-0.10	-0.25	-0.60
11	-0.40	-0.60	-0.65	-0.60
12	-0.40	-0.60	-0.65	-1.15

Recommended Section Pressure Coefficients, Model A5

Section	Azimuth Angle			
	0	30	60	90
1	-0.60	-0.90	-1.05	-0.85
2	-0.60	-0.55	-0.40	-0.30
3	-0.50	-0.60	-0.45	-0.30
4	-0.50	-0.60	-0.80	-0.85
5	-0.55	-0.80	-0.95	-0.85
6	-0.55	-0.10	0.50	0.60
7	-0.50	-0.40	0.25	0.60
8	-0.50	-0.55	-0.75	-0.85
9	-0.35	-0.90	-1.20	-1.05
10	-0.35	-0.05	-0.20	-0.45
11	-0.50	-0.75	-0.65	-0.45
12	-0.50	-0.55	-0.75	-1.05

Recommended Section Pressure Coefficients, Model A6

Section	Azimuth Angle			
	0	30	60	90
1	-0.70	-0.90	-0.80	-0.80
2	-0.70	-0.55	-0.30	-0.25
3	-0.65	-0.75	-0.40	-0.25
4	-0.65	-0.80	-0.80	-0.80
5	-0.60	-0.90	-0.80	-0.80
6	-0.60	-0.15	0.50	0.65
7	-0.60	-0.45	0.35	0.65
8	-0.60	-0.75	-0.80	-0.80
9	-0.35	-0.85	-0.90	-1.00
10	-0.35	-0.05	-0.15	-0.45
11	-0.55	-1.00	-0.65	-0.45
12	-0.55	-0.70	-0.70	-1.00



Recommended Section Pressure Coefficients, Model B6

Section	Azimuth Angle			
	0	30	60	90
1	-0.80	-1.00	-0.75	-0.85
2	-0.80	-0.75	-0.55	-0.45
3	-0.70	-0.85	-0.60	-0.45
4	-0.70	-0.90	-0.80	-0.85
5	-0.75	-0.90	-0.80	-0.80
6	-0.75	-0.10	0.50	0.75
7	-0.65	-0.10	0.40	0.75
8	-0.70	-0.70	-0.80	-0.80
9	-0.30	-1.00	-1.00	-0.80
10	-0.30	-0.10	-0.25	-0.45
11	-0.60	-0.80	-0.80	-0.45
12	-0.60	-0.75	-0.80	-0.80

Recommended Section Pressure Coefficients, Model C6

Section	Azimuth Angle			
	0	30	60	90
1	-0.75	-1.10	-0.85	-1.10
2	-0.75	-0.90	-0.65	-0.85
3	-0.70	-0.90	-0.80	-0.85
4	-0.70	-1.05	-0.90	-1.10
5	-0.70	-1.00	-0.80	-1.00
6	-0.70	-0.10	0.50	0.85
7	-0.70	-0.10	0.50	0.85
8	-0.70	-0.85	-0.85	-1.00
9	-0.25	-1.05	-1.10	-1.15
10	-0.25	-0.10	-0.30	-0.65
11	-0.55	-0.85	-1.00	-0.65
12	-0.55	-0.85	-0.85	-1.15

Recommended Section Pressure Coefficients, Model D

Section	Azimuth Angle			
	0	30	60	90
1	-0.65	-1.05	-1.10	-0.75
2	-0.65	-0.75	-0.55	-0.10
3	-0.55	-0.70	-0.50	-0.10
4	-0.55	-0.75	-1.00	-0.75
5	-0.60	-0.65	-0.70	-0.80
6	-0.60	-0.35	0.15	0.50
7	-0.55	-0.40	0.05	0.50
8	-0.55	-0.50	-0.75	-0.80
9	-0.70	-1.35	-1.35	-0.90
10	-0.70	-0.55	-0.75	-0.60
11	-0.55	-0.70	-0.70	-0.60
12	-0.55	-0.60	-0.70	-0.90

Recommended Section Pressure Coefficients, Model F

Section	Azimuth Angle			
	0	30	60	90
1	-0.50	-0.60	-0.50	-0.65
2	-0.50	-0.35	-0.35	-0.95
3	-0.75	-0.30	-0.25	-0.55
4	-0.55	-0.50	-0.50	-0.55
5	-0.65	-0.70	-1.00	-0.95
6	-0.65	-0.75	1.05	0.65
7	-0.55	-0.90	0.85	0.70
8	-0.75	-1.00	-0.70	-0.70
9	-0.65	-0.35	-0.40	-0.50
10	-0.65	-0.05	-0.10	-0.80
11	-0.80	-0.10	-0.40	-0.65
12	-0.40	-0.25	-0.10	-0.65
13	-0.60	-0.60	-0.90	-0.80
14	-0.60	-0.75	-1.05	-0.50
15	-0.40	-0.85	-0.80	-0.60
16	-0.80	-1.05	-0.65	-0.60

**Appendix C**

**SECTION PEAK PRESSURE COEFFICIENTS**

PEAK PRESSURE COEFFICIENTS IN EACH SECTION, MODEL A1

Section	Azimuth Angle			
	0	30	60	90
1	-0.80	-1.26	-3.06	-1.21
2	-0.80	-1.20	-2.13	-1.17
3	-0.50	-0.83	-1.08	-1.17
4	-0.50	-0.85	-1.11	-1.21
5	-0.56	-0.89	-0.91	-1.00
6	-0.56	-0.07 (0.18)	0.69	0.56
7	-0.41	-0.32 (0.02)	0.38	0.56
8	-0.41	-0.46	-0.98	-1.00
9	-0.98 (0.78)	-1.62	-2.33	-1.77
10	-0.98 (0.78)	-0.93 (0.78)	-1.76 (0.83)	-1.49 (0.45)
11	-0.59	-1.07	-1.36	-1.49 (0.45)
12	-0.59	-0.65	-0.60	-1.77

PEAK PRESSURE COEFFICIENTS IN EACH SECTION, MODEL A3

Section	Azimuth Angle			
	0	30	60	90
1	-0.80	-1.45	-2.44	-1.17
2	-0.80	-1.30	-1.79	-1.11
3	-0.54	-0.91	-1.06	-1.11
4	-0.54	-0.91	-1.15	-1.17
5	-0.60	-0.93	-1.09	-1.00
6	-0.60	-0.09 (0.11)	0.69	0.57
7	-0.44	-0.32 (0.09)	0.34	0.57
8	-0.44	-0.41	-0.89	-1.00
9	-0.96 (0.78)	-2.04	-2.20	-1.87
10	-0.96 (0.78)	-1.04 (0.68)	-1.65 (0.83)	-1.43 (0.45)
11	-0.61	-0.96	-1.36	-1.43 (0.45)
12	-0.61	-0.85	-0.76	-1.87

PEAK PRESSURE COEFFICIENTS IN EACH SECTION, MODEL A6

Section	Azimuth Angle			
	0	30	60	90
1	-0.93	-1.48	-0.94	-0.82
2	-0.93	-1.20	-0.81	-0.83
3	-0.70	-1.16	-0.78	-0.83
4	-0.70	-1.12	-0.82	-0.82
5	-0.64	-0.87	-0.77	-0.82
6	-0.64	-0.17 (0.06)	0.56	0.67
7	-0.57	-0.43	0.46	0.67
8	-0.57	-0.75	-0.76	-0.82
9	-1.07 (0.76)	-1.69	-2.06	-1.57
10	-1.07 (0.76)	-0.93 (0.82)	-1.06 (0.89)	-1.17 (0.59)
11	-0.78	-1.88	-1.33 (0.04)	-1.17 (0.59)
12	-0.78	-0.80	-0.80	-1.57



PEAK PRESSURE COEFFICIENTS IN EACH SECTION, MODEL B6

Section	Azimuth Angle			
	0	30	60	90
1	-1.11	-1.55	-0.83	-0.94
2	-1.11	-1.32	-0.83	-0.98
3	-0.76	-1.28	-0.96	-0.98
4	-0.76	-1.30	-0.94	-0.94
5	-0.79	-0.91	-0.81	-0.82
6	-0.79	-0.11	0.64	0.76
7	-0.57	-0.13 (0.04)	0.48	0.76
8	-0.57	-0.71	-0.79	-0.82
9	-1.17 (0.76)	-2.85	-1.66	-0.91
10	-1.07 (0.76)	-0.93 (0.82)	-1.06 (0.89)	-1.17 (0.61)
11	-0.78	-1.23	-0.83	-1.15 (0.61)
12	-0.78	-1.17	-1.74	-0.91

PEAK PRESSURE COEFFICIENTS IN EACH SECTION, MODEL C6

Section	Azimuth Angle			
	0	30	60	90
1	-0.96	-1.74	-0.91	-1.54
2	-0.96	-1.42	-1.26	-1.46
3	-0.72	-1.37	-1.12	-1.46
4	-0.72	-1.46	-1.14	-1.54
5	-0.69	-1.02	-0.83	-1.05
6	-0.69	-0.09	0.56	0.90
7	-0.71	-0.29 (0.10)	0.64	0.90
8	-0.71	-0.89	-0.90	-1.05
9	-1.32 (0.76)	-2.56	-2.20	-1.85
10	-1.32 (0.76)	-1.34 (0.94)	-1.63 (0.96)	-1.65 (0.46)
11	-0.74	-1.50	-2.85	-1.65 (0.46)
12	-0.74	-1.27	-0.90	-1.85

PEAK PRESSURE COEFFICIENTS IN EACH SECTION, MODEL D

Section	Azimuth Angle			
	0	30	60	90
1	-1.36	-3.04	-1.23	-0.88
2	-1.36	-2.54	-1.63 (0.04)	-0.60 (0.48)
3	-1.06	-1.32	-1.20	-0.60 (0.48)
4	-1.06	-1.41	-1.34	-0.88
5	-0.78	-0.74	-0.73	-0.78
6	-0.78	-0.39	0.22	0.60
7	-0.58	-0.67	-0.22 (0.28)	0.60
8	-0.58	-0.43	-0.72	-0.78
9	-1.36 (0.38)	-3.04	-1.74	-1.71
10	-1.36 (0.38)	-2.54	-2.11 (0.22)	-1.76 (0.47)
11	-1.06	-1.00	-1.08	-1.76 (0.47)
12	-1.06	-1.05	-0.93	-1.71

**Appendix D**  
**EXAMPLE WIND LOAD CALCULATIONS**

PROBLEM:

Find wind and anchor loads on the center section of a series 50 clamsmeter w/7 bays as a function of velocity. Assume 90-degree azimuth angle.

SOLUTION:

From NAVFAC DM-2.2, wind loads on structures are calculated from,

$$q = 0.00256 C_h C_p v^2$$

where:  $q$  = load (in  $\text{lb/ft}^2$ )

$C_h$  = height correction factor ( = 1 for  $h < 30$  ft)

$C_p$  = pressure coefficient

$v$  = wind velocity (mph)

For the structure under study, the overall dimensions are,

$h$  = 24 ft, peak; (10-ft eave)

$d$  = 61 ft

$l$  = 140.5 ft (87.5-ft center section)

then,

$$l/d = 2.30, \quad h/d = 0.39$$

from the models tested, model A3 was,

$$l/d = 2.26, \quad h/h = 0.38$$

The appropriate section average and peak pressure coefficients are found in Appendixes B and C. They are:

Section	$C_p$	$C_p$ (max)
1. Windward side	0.55	0.57
2. Windward roof	-0.40	-1.11
3. Leeward roof	-1.00	-1.17
4. Leeward side	-0.95	-1.00

Substitution into the wind load equation gives,

Section	$q$	$q$ (max)
1	$0.0014V^2$	$0.0015V^2$
2	$-0.0010V^2$	$-0.0028V^2$
3	$-0.0026V^2$	$-0.0030V^2$
4	$-0.0026V^2$	$-0.0030V^2$

These are plotted in Figures D-1 and D-2.

The total section loads are found from,

$$f_T = q A$$

where  $f_T$  = total section load

$A$  = section area

for this structure,

Section	Area (ft <sup>2</sup> )	Ft (lbs)
1	894.25	1.26V <sup>2</sup>
2	2794.75	-2.86V <sup>2</sup>
3	2794.75	-7.15V <sup>2</sup>
4	894.25	-2.17V <sup>2</sup>

These loads are plotted in Figure D-3.

To calculate anchor loads, the section loads per foot of length are given above. These can be resolved to concentrated loads acting at the midpoint of each section, or shown in Figure D-4 (assuming the frame is pinned on the windward side and simply-supported on the leeward side.)

$$\Sigma f_x = 0 = -h_1 + 0.014^2 - 0.0143v^2 + 0.0364v^2 + 0.0240v^2;$$

$$h_1 = 0.0601v^2$$

$$\Sigma F_y = 0 = -v_1 - 0.003v^2 + 0.0294v^2 + 0.0746v^2 + 0.0051v^2 - v_2;$$

$$v_1 + v_2 = 0.1061v^2$$

$$\begin{aligned} \Sigma M_a = 0 = & (0.003)(1.06)v^2 + (0.014)(4.99)v^2 - (0.0143)(17)v^2 \\ & - (0.0294)(16.48)v^2 - (0.0746)(44.52)v^2 + (0.0364) \\ & (17)v^2 - (0.0051)(5.94)v^2 + (0.0240)(4.99)v^2 + 61v_2; \end{aligned}$$

$$v_2 = 0.0581v$$

from (2),

$$v_1 = 0.0480v^2$$

On the windward side,

$$v_T = [(0.0480v^2)^2 + (0.0601v^2)^2]^{1/2} = 0.0769v^2$$

The total anchor loads are,

$$v_T \text{ (windward)} = 6.7288v^2$$

$$v_T \text{ (leeward)} = 5.0838v^2$$

These are plotted in Figure D-5.

The total required anchor capacities are plotted in Figure D-6.

# SECTION LOADS VS. WIND VELOCITY

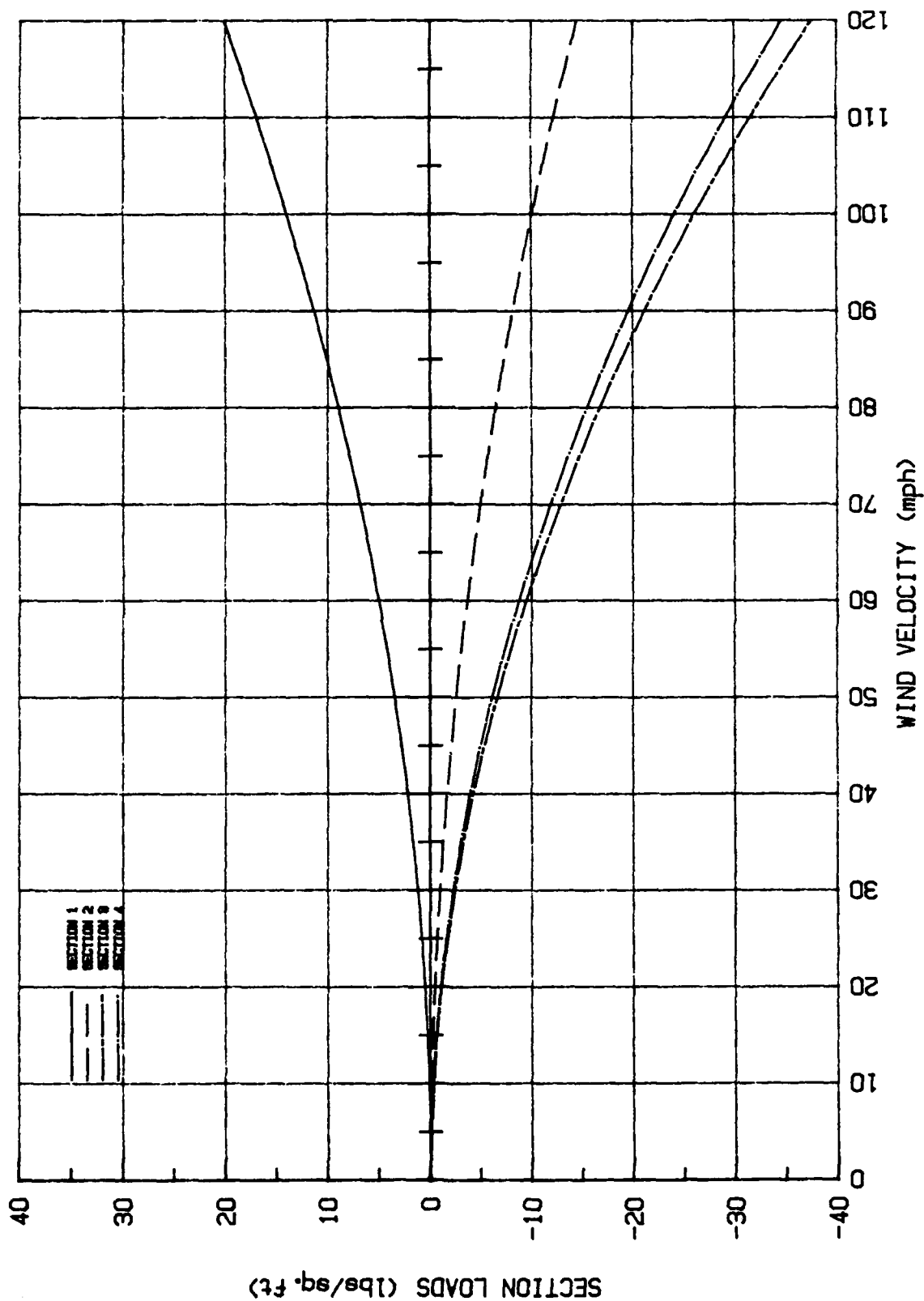


Figure D-1. Section loads versus wind velocity.



# PEAK SECTION LOADS VS. WIND VELOCITY

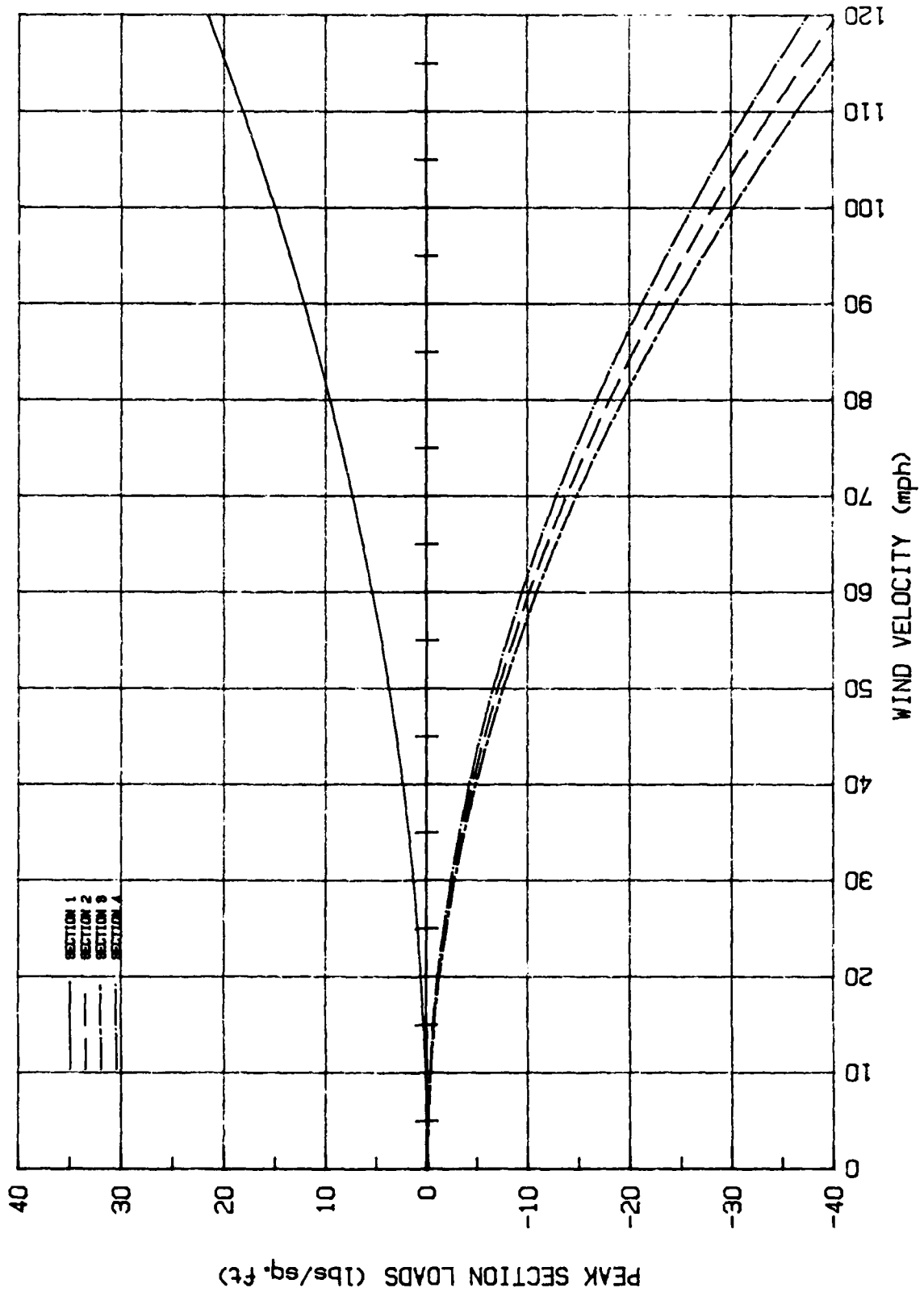


Figure D-2. Peak section loads versus wind velocity.

# TOTAL SECTION LOADS VS. WIND VELOCITY

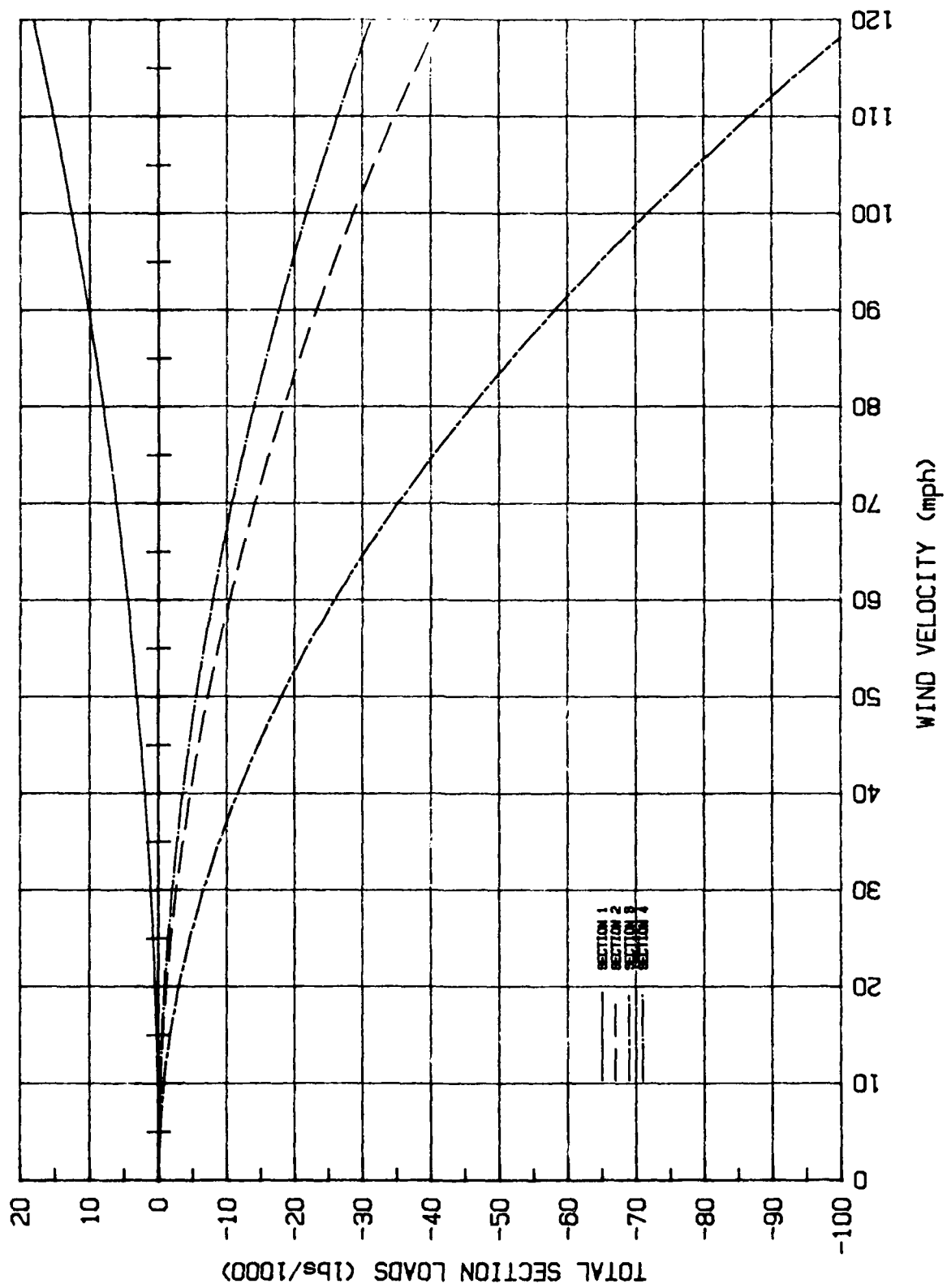


Figure D-3. Total section loads versus wind velocity.

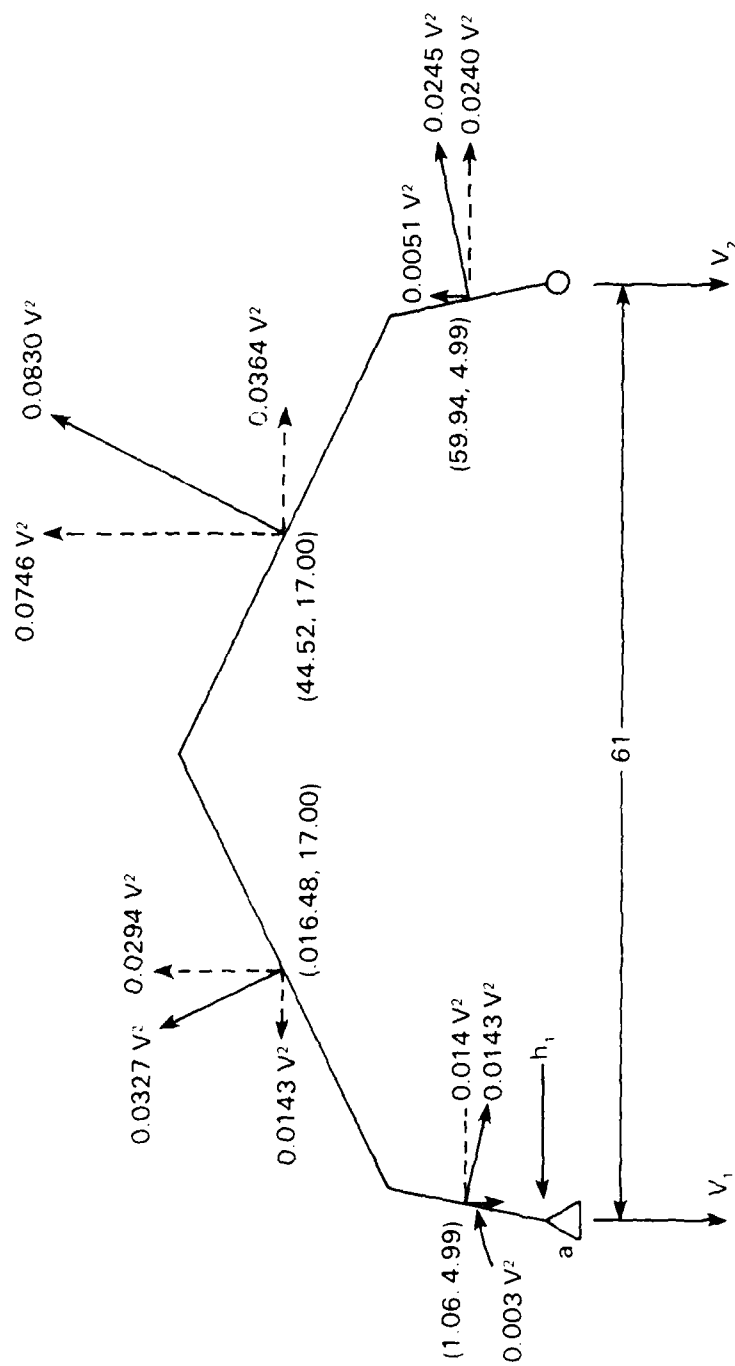


Figure D-4. Load concentrations.

# TOTAL ANCHOR LOADS VS. WIND VELOCITY

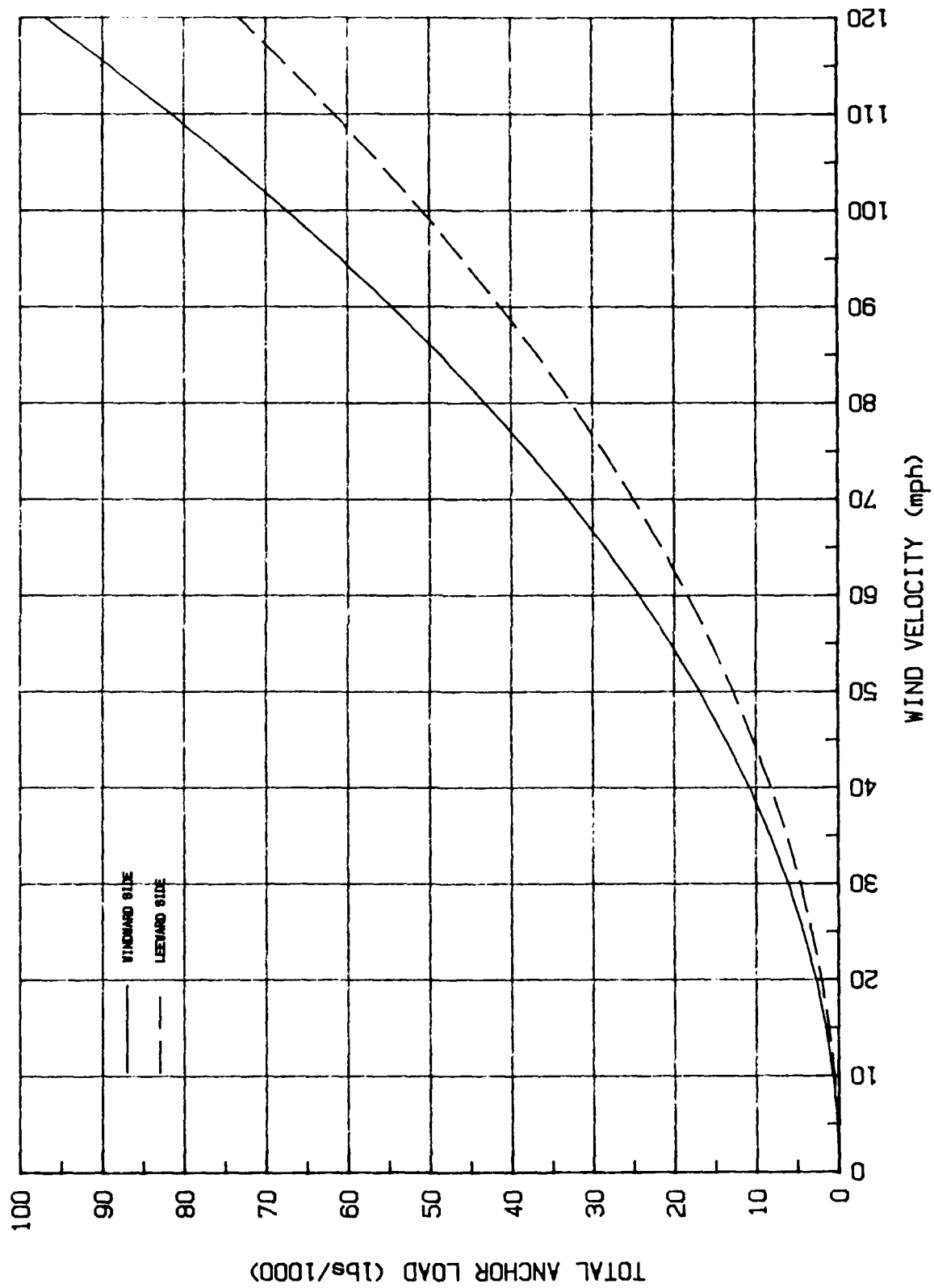


Figure D-5. Total anchor loads versus wind velocity.

REQUIRED ANCHOR CAPACITY

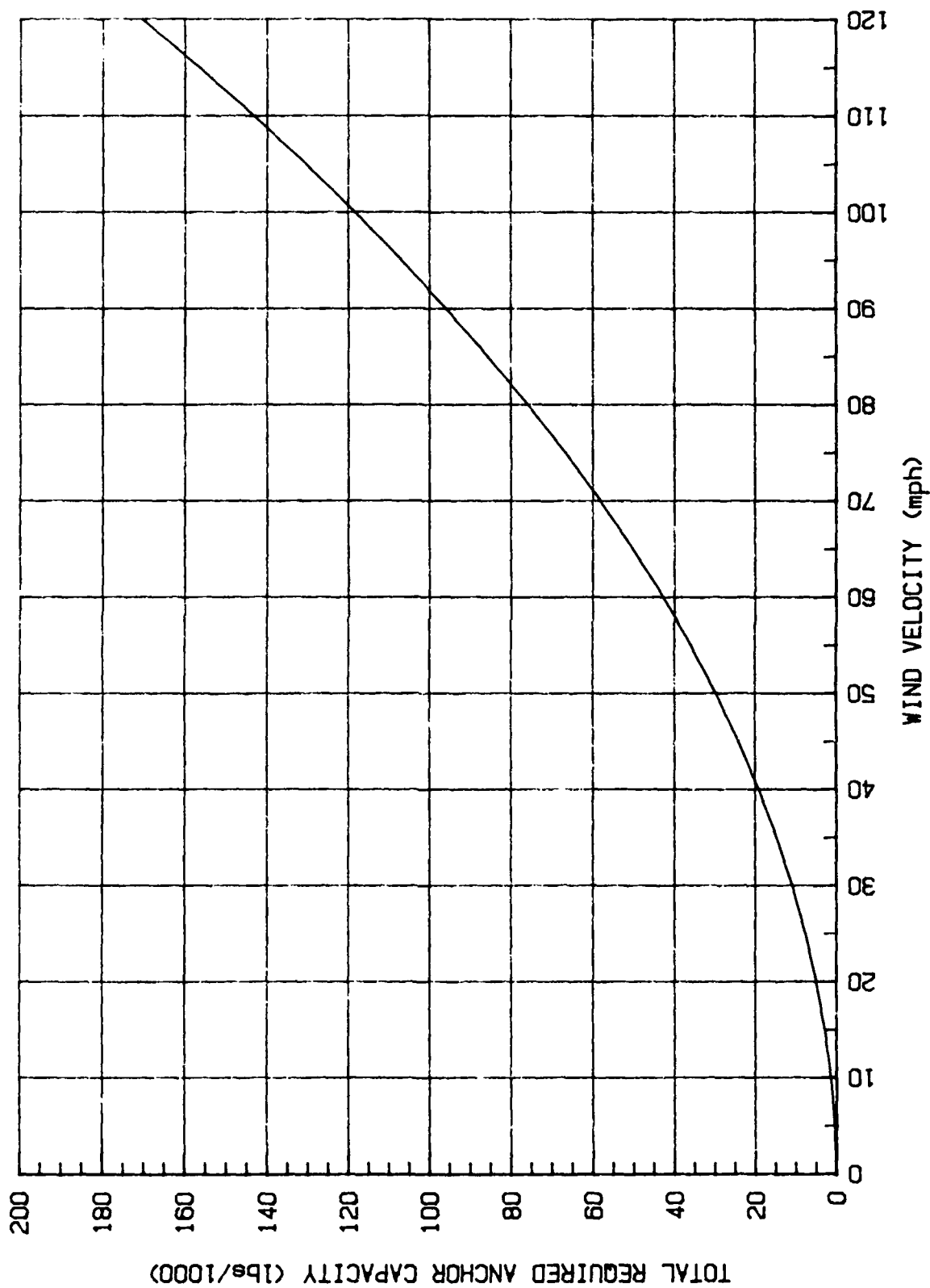


Figure D-6. Required anchor capacity.

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